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Effects of Long-Term Flow Variation on Microhydropower Energy Production in Pressure Reducing Valves in Water Distribution Networks

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1   **Effects of Long-term Flow Variation on Micro-Hydropower Energy Production in Pressure**

2   **Reducing Valves in Water Distribution Networks.**

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4

5   **ABSTRACT**

6   Incorporating micro-hydropower (MHP) turbines within water supply networks has the potential to

7   improve the economic and environmental sustainability of the sector. However, long-term flow and head

8   variations in water networks is a key risk factor which increases turbine performance uncertainty in the

9   medium-to-long term, potentially impacting on the investment payback period. Using high-resolution

10 historical flow and head data across a number of pressure reducing valve sites in water networks in

11 Ireland, this study presents an assessment of the impact of flow and head variations on turbine efficiency

12 and power output over a twenty year period. Results indicated that pumps-as-turbines (PATs) represent

13 a viable low-cost option over the long-term, at sites with smaller power output potential. Where flow

14 and head rates displayed considerable fluctuation, the integration of a two-PAT configuration could

15 improve operating efficiency and maximise power output. This design strategy opens up the opportunity

16 to conduct energy recovery from sites which may previously have been considered unsuitable for MHP.

17

20

1   Introduction

22 A continuous high quality water supply is a vital facet of effective societal and economic development

23 across nations. Such continuity of service is predicated on sustained energy security and affordability

24 into the future. The water industry is particularly vulnerable within this context as water abstraction,

25 treatment and distribution are energy intensive processes. Globally, 2-3% of total energy consumption is

26 associated with pumping and treating water (Kwok et al. 2010). The UK water industry for example,

27 utilises approximately 3% of total energy demand (Environment Agency, 2009) emitting over 5 million

28 tonnes of CO_2 emissions annually (DEFRA, 2008). Concurrently, the overall cost of water provision is

29 rising due to increased energy costs (Zilberman et al. 2008). In Ireland, water service provision costs

30 have been increasing by approximately 7.5% per year since 2007 and key drivers include higher capital

31 investment requirements, rising energy costs together with more stringent regulatory compliance in

32 terms of both national and European Union (EU) legislation (DoEHLG, 2010). Accordingly, there is a

33 pressing need to achieve greater efficiencies across water infrastructure in conjunction with the

34 integration of economically viable renewable energy technology solutions.

35   Opportunities exist for energy efficiencies across the entire water supply chain. A breakdown of

36 energy demand across water service provision reveals that water distribution accounts for 45% of total

37 energy consumption (Daigger, 2009). Many water utilities are now incorporating renewable

38   diversification with a range of energy applications including: hydropower, wind turbines, solar power,

39   the generation of energy in wastewater treatment facilities (Kwok et al. 2010; UKWIR 2010).

40   In terms of hydropower, large-scale installations are widespread on a global scale, yet, micro-

41   hydropower (MHP) at various water infrastructure locations has experienced limited market penetration

42   to date (Gaius-obaseki 2010). Vilanova and Balestieri (2014) note that the use of hydraulic turbines

43   inserted within water distribution networks represents one of the most complex forms of energy

44   recovery in water supply systems. There is a need to address identified barriers to the uptake of MHP in

45   an effort to strengthen the investment case for greater acceptance of this technology in the water industry

46   (McNabola et al., 2014a). One such barrier is long-term network flow and head variation and their

47   potential impact on the operational efficiency of turbines.

48   Turbines are designed for a relatively stable flow rate, yet flow variation can occur diurnally,

49   seasonally and over the long-term which can impact on efficiency and thus capital payback (Sitzenfrei

50   and Rauch, 2015). Carravetta et al. (2014a) highlight the importance of flexibility within an energy

51   production system given that operating conditions can vary due to network flow variation during its life

52   cycle. Climate change, population growth, leakage rates, water pricing and economic activity have all

53   been shown to have an impact on long-term flow and head variations in water distribution networks

54   (Corcoran et al., 2016).

55   Considering the initial high capital investment requirement, there is a need to ensure the long-term

56   viability of a MHP installation. Accordingly, this paper aims to investigate long-term fluctuations in

57   flow rates and head over time at three potential hydropower locations within the water supply network

58   of Dublin City (Ireland). The viability and operational resilience of three turbine options including a

59   Kaplan and pumps-as-turbines (PATs) are assessed and outcomes are compared in terms of energy

60   recoverable, payback periods and gross income. The paper concludes with engineering design

61   recommendations regarding future MHP installations in light of potential increases in flow/head

62   variability into the future.

2   Hydropower Energy Recovery in Water Networks

The potential for energy recovery via MHP has been identified within water supply networks at points of high flow or surplus hydraulic head which otherwise needs to be dissipated for pressure management purposes (Vicente et al. 2016). Such applications include flow control valves, pressure reducing valves (PRVs), storage/service reservoirs, break pressure tanks and wastewater treatment plants (Williams et al. 1998; Saket 2008; Gaius-obaseki 2010; Corcoran et al. 2012; Power et al. 2014; McNabola et al. 2014a; Samora et al. 2016). This excess energy can be recovered and converted into electricity without reducing the level of service to customers.

Specifically, pressure reducing valves (PRVs) have been identified as a large untapped resource and Carravetta et al. (2014b) note that the number of PRVs is increasing across networks as they can reduce leakage and delay the need for expensive rehabilitation works. Gaius-obaseki (2010) states that up to 85% of wasted energy can be recovered through replacement of a PRV with a turbine or alternatively through installing a turbine and a PRV in parallel. However, technological and economic viability barriers exist which to date have prevented the exploitation of this potential energy saving.

Many studies have identified the potential for pumps-as-turbines (PATs) to produce energy in water networks (Williams 1996; Ramos and Borga 1999; García et al. 2010; Carravetta et al. 2012; Carravetta et al. 2014b; Fecarotta et al. 2015). PATs, where a water pump is run in reverse, have a cost advantage over conventional turbines for small scale energy generation, as a wide range of pump sizes are mass produced. Furthermore, they are easy to install (Williams 1996) and spare parts are widely available (Agarwal 2012). In contrast, hydraulic turbines are considerably more expensive due to fact that they are specifically designed for each site. However, they display greater efficiencies over a wider range of flow and head rates when compared to PATs. Additionally, PATs do not possess a regulation device so this must be included during installation where pressure control is required (Carravetta et al. 2014a). To date, real scale installation of hydraulic turbines and PATs within water distribution networks remains somewhat limited. There are evident challenges when installing either turbine option within the small

88   distribution network setting, specifically the smaller power potential across sites and high variability in

89   hydraulic characteristics when compared to larger transmission pipelines (Giugni et al. 2014; Carravetta

90   et al. 2014b). Furthermore, previous research has established that a mere 10% change in flow rate at a

91   small sized plant can increase the payback period and render a MHP project unsuitable (McNabola et al.

92   2014b).

93   Given that MHP installations typically have an investment payback period of 10 years, there is a

94   need to assess future flow uncertainties into the medium-to-long term. Research regarding the impact of

95   demand uncertainty and long-term flow variation specifically on turbine efficiency is relatively limited.

96   Sitzenfrei and von Leon (2014) utilised ten years of hourly water consumption data in a simulation

97   model for the design and optimisation of a small hydropower system testing various turbine sizes.

98   Additional research involved the use of this long-time simulation model to analyse the effects on a small

99   hydropower system in which a control mechanism for the device was optimised in order to maximise

100 profits (Sitzenfrei et al. 2014). More recently, Sitzenfrei and Rauch (2015) assessed the impact of

101 different future population and demand scenarios on the performance of a small Pelton hydropower

102 system in Austria and the authors stressed that disregarding both long-term demand patterns and demand

103 uncertainty hinders the attainment of a realistic evaluation of potential profits. Similarly, Colombo and

104 Kleiner (2011) highlight the importance of considering changes in demand over time. Their study

105 probabilistically analysed the feasibility of energy recovery via micro turbines and identified that diurnal

106 and seasonal demand fluctuations can significantly impact project return.

107 The optimal choice of turbine is dictated by the flow and pressure range of the site (Gaius-obaseki,

108 2010) and high variability in user demand can significantly impact turbine suitability. Sitzenfrei et al.

109 (2014) comment that within a 20-year period, water infrastructure and small hydropower installations

110 can be significantly impacted by population dynamics and water use. Limited research has analysed the

111 performance and operational efficiency of turbines using historical long-term flow data, and no

112 investigations have examined the long-term performance of PATs to date. Accordingly, this study aims

113 to fill this gap through assessment of historical flow and head variation using up to twenty years of high-

114 resolution data across three PRV sites within a water distribution network and analysing the resulting

115 impact on available volumes of water for energy production across a number of different turbine design

116 scenarios. A near-optimal MHP design strategy for small capacity sites, in terms of improving turbine

117 efficiency performance over the long-term, is subsequently developed and discussed.

118

119

120 **3   Methodology**

121 *3.1 Simulation of long-term turbine performance*

122 The study firstly analyses the extent of long-term fluctuations in flow and head across three PRV sites

123 over a period of up to 20 years. It was anticipated that due to population and economic growth, user

124 demand and thus flow rates would change significantly over the time period across the three sites.

125 The first year of data in each historical record was utilised to establish a design flow for a

126 hypothetical turbine installation at each location, assuming year one in the historical dataset represented

127 the present day. It is common practice in the design of MHP installations in both run-of-river and water

128 network settings to establish the turbine design flow, Q_0, based on the average flow from one year of

129 flow data. However as this paper aims to demonstrate, such practices are fraught with inaccuracies, most

130 particularly in water distribution.

131 The paper presents a theoretical simulation of the potential performance of varying turbine design

132 options at the three PRV sites over the intervening years in the historical record (16-19 years), assuming

133 that these data represent future flow rates. Turbine efficiencies were evaluated over this long-term period

134 in response to flow and head variation. Total reductions in CO_2 emissions were also estimated.

135

136 *3.2 Turbine Design Scenarios*

137 The three turbine design scenarios investigated are displayed in Figure 1. Firstly, a traditional Kaplan

138 turbine was selected due to its wide high-efficiency range (see Figure 3) and suitability for the low-head

139 and high-flow conditions of the three PRVs. Secondly, a single PAT was assessed at each site. Whilst a

140 PAT possesses a narrower high-efficiency range, it is considerably lower in cost when compared to a

141 conventional hydraulic turbine.

142

143 **Figure 1**. Installation schemes of three turbine scenarios; a traditional Kaplan turbine, PAT and two

144 PATs in parallel (Adapted from Carravetta *et al.* (2012)).

145 Considering this low cost, a third scenario incorporated two differently sized PATs in which flow

146 would be directed through either the larger PAT with a design flow based on the average flow rate in

147 year 1 or alternatively through the smaller sized PAT designed for 50% less than that design flow.

148 Therefore, the optimal choice of PAT in scenario three was dependent on the incoming flow rate and

149 flow was switched to the smaller PAT when this would produce a higher power output. This two-PAT

150 scenario was included in order to increase efficiency and power generation potential. Both PAT systems

151 also included the concept of a hydraulic regulation device to control downstream pressure as described

152 by Carravetta et al. (2014a). All turbine scenarios incorporated a by-pass system to prevent disruption to

153 the supply service in the event of maintenance requirements or failure of the turbine.

154

155 *3.3 Case Study Area - Dublin*

156 This study builds on previous research regarding the MHP energy recovery potential of the Dublin water

157 supply network (Corcoran et al. 2012, 2013, 2016; McNabola et al. 2014b) through analysis of a subset

158 of PRV sites in the network (see Figure 2). In this paper, the viability of three turbine configurations

comprising either a hydraulic turbine or a PAT is investigated with the aim of exploring their operational efficiencies and economic suitability over the long-term.

Figure 2. Map of the Dublin region displaying the location of the three pressure reducing valves used in the case study.

High resolution telemetry data of flow and head at 15 minute intervals collected by Dublin City Council was utilised for simulation of turbine performance across three PRV sites: Thomas Court; Blackhorse Bridge; and Merrion Gates, over 20 years (up to 700,800 measurements). These sites were selected as they possessed different flow and head characteristics together with varied power output potential, as outlined in Table 1. Head data comprised both inlet and outlet head readings. The availability of data varied across sites ranging from 17 years up to 20 years (1993 to 2013).

Thomas Court was located on a section of the network which feed a large industrial user of water. This user was the largest water user in Dublin and required a high flow rate. High flow and pressure was delivered to this location to meet processing needs. The Blackhorse bridge PRV was located in a mainly residential area, while Merrion Gates was located adjacent to a large hospital. Each site served quite differing water demand types, which partly explains the reasons for differing head and flow values shown in Figure 4. In addition to this, each of the 3 values are located in differing sections of Dublin, one in the city centre, one in the south and one in the north-west. The cumulative demands from source to supply in each area was different.

3.4 Simulation of Power Output Potential and Estimation of Return of Investment

180   The three sites differed regarding their estimated power potential. Table 1 displays the average flow rate,

181   head and estimated power potential across the PRVs.

182

183   **Table 1.** An overview of flow, head and power output estimates for three PRVs in Dublin. Power

184   estimates were based on varying Kaplan turbine efficiencies, where the turbine design flow/head was

185   assumed to be the average of the data from year 1 of the record.

186

187   The potential power output at each site was simulated for every 15 minute interval within the 20-year

188   dataset using equation (1), where P represents the power output (kW), Q is the flow rate through the

189   turbine (m³/s), ρ is fluid density (kg/m³), g is acceleration due to gravity, H is the available head (PRV

190   head drop) at the turbine (m) and e_o represents the overall system efficiency.

$$P = Q\rho gHe_o \tag{1}$$

192   Therefore flow and head varied according to their measured input values (head was taken as the

193   difference between input and output head at the PRV i.e. available excess head). Overall system

194   efficiency included a variable turbine efficiency value together with generator and transmission loss

195   efficiencies estimated to be 85% and 98% respectively (Power et al., 2014). Turbine rotational speed and

196   therefore efficiency varied according to the extent of deviations in the instantaneous flow and head

197   measurements from their design values (selected as the average flow and average head in year 1 of the

198   data records). Turbine efficiency curves, adapted from Corcoran et al. (2013) and Ørke (2010), were

199   used to quantify these changes as shown in Figure 3. A sixth-degree polynomial equation was fitted to

200   data in each efficiency curve and used to estimate of overall system efficiencies for each turbine option

201   according to Equation 2. In terms of historical demands and turbine design, the average flow rate over

202   the first year of available data at each PRV site was utilised as the design flow criteria for each turbine

203   option. For the two-PAT scenario, the design flow for the second smaller PAT was chosen as 50% less

204   than the average annual flow rate.

205

206   **Figure 3**. Overall system efficiency curves for the Kaplan turbine and PAT, assuming generator and

207   transmission loss efficiencies of 85% and 98% respectively.

208

209                                    $$e_0 = e_{turbine} \times e_{generator} \times e_{transmission} \qquad (2)$$

210   Where $e_{turbine}$ is the instantaneous turbine efficiency; $e_{generator}$ is the generator efficiency; and $e_{transmission}$ is

211   the transmission efficiency.

212   In terms of assessing economic feasibility, a payback period approach was applied where the payback

213   period equals the investment cost divided by the net annual revenue (ESHA, 2004). In general, MHP

214   projects which exceed a payback period of 10 years are not considered viable by water utilities

215   (McNabola et al., 2014b). The overall costs of an MHP installation comprise the initial installation costs

216   (design, construction, installation and commissioning) and subsequent operation and maintenance costs.

217   Generally, MHP projects require large upfront investment costs with low recurring costs thereafter.

218   Installation costs for an MHP turbine are mainly site specific and can differ depending on the amount of

219   civil works needed and proximity to the grid. It has been estimated that capital costs for the installation

220   of micro-hydropower are in the range of £3,000 to £6,000 per kW installed and costs decrease with an

221   increase in capacity or for higher head turbines (Gaius-obaseki, 2010). Similarly, MHP turbine

222   installation costs in America are estimated to be in the region of $3,500-$7,000/kW whilst maintenance

223   costs are approximately $2,000 annually (Colombo and Kleiner, 2011). In the present study, installation

224   costs for the Kaplan turbine were estimated using an empirical formula developed by Ogayar et al.

225   (2009) based on power output and hydraulic head (Equation 3). The cost per kW for a PAT was

226 estimated at €350/kW according to previous research undertaken by Carravetta et al. (2013), as no cost-
227 power-head function is currently available for PATs.

228

229 $$Kaplan\ Cost = 31196.P^{0.41662}.H^{-0.113901}$$ (3)

230 Where *Kaplan cost* represents the euro value of electromechanical equipment; P is the power output
231 (kW); and H is the head (m). Both of these costs estimates relate to the electromechanical equipment
232 only and do not incorporate civil construction works. In the present study it was assumed that the turbine
233 cost represented 30% of total installation costs, signalling that civil and construction works amounted to
234 70% of total expenditure, as per previous research findings (Gallagher et al. 2015). An additional fixed
235 maintenance costs of €1,496 ($2,000) per annum (Colombo and Kleiner 2011) was also incorporated in
236 the economic analysis.

237 It was assumed that the electricity generated would be utilised on site rather than connecting to the
238 grid, thus reducing the total investment requirement. This option has previously been found to be more
239 economically advantageous in Ireland due to low REFIT rates for MHP (Corcoran et al. 2013).
240 Accordingly, annual power generation was multiplied by the end user industrial price of electricity for
241 2013 of €0.137/kW, in order to establish annual electricity savings (Eurostat 2014b). In terms of the
242 environmental benefit, equivalent CO_2 emissions from electricity generation were calculated based on
243 2013 figures of 528 g per kWh in Ireland (SEAI, 2013).

244

245 **4 Analysis and Results**

246 *4.1 Long-term Flow Variation*

Average annual flow and head data for each PRV site are displayed in Figure 4. The analysis revealed considerable variability between sites and highlights the influence of local water demands in each area. The Merrion Gates PRV, for example, served a nearby hospital which would possess a different demand pattern when compared to flow feeding residential or commercial districts. Whilst it would be reasonable to forecast gradual increases in demand due to expected economic and population growth, the data indicate that average flow rates decreased substantially during the 1990s. Given that turbines are designed (and would be selected) according to a particular performance band, this reduction in flow rate could impact turbine efficiency and thus energy recovery. During the 2000s, a general increasing trend in demand was evident in line with the Irish economic boom period but a second prolonged decrease was observed at the smallest PRV site, Merrion Gates. Such deviation creates difficulties when attempting to optimise the turbine design flow. In contrast to flow rates, long-term variations in head were less extreme across sites. Figures S1 to S2 in the supplementary materials sections illustrates the variation in power output using the 3 turbine options considered here.

Figure 4. Long-term flow and head variation across three PRV sites. a) Thomas Court, 145 kW (1994 - 2013); b) Blackhorse Bridge, 75 kW (1996 - 2013) and c) Merrion Gates, 12.5 kW (1993 - 2013).

4.2 Turbine Comparisons: Energy Recovery Potential and Investment Payback

The impact of turbine selection on energy recovery and payback periods is presented in Table 2. Estimated gross income was calculated assuming an annual power generation based on the design year (i.e. performance was projected over the 20-year period based on a design flow from year 1 only, as would be standard practice). Subsequently, actual gross income was determined, reflecting analysis of

269 the true fluctuations in power generation over the subsequent 16-19 years for each site. For the two-PAT

270 scenario, the percentage of time the smaller sized PAT was in use over the period is also shown.

271 Findings revealed that significant power generation capacity exists across each of the scenarios. The

272 Kaplan produced the greatest amount of energy across all sites, owing to its higher overall efficiency

273 compared to the PAT (as illustrated in Figure 3). However, the Kaplan cost approximately 25% more to

274 install than either a single PAT or two PATs system. This is in line with previous research which also

275 highlighted the lower cost of PATs when compared to conventional turbines (Williams 1996; Nautiyal

276 and Varun 2010). Furthermore, the cost difference was greater at the site with the lowest power output

277 potential (the Kaplan turbine cost 29% more than a PAT).

278

279 **Table 2**. Estimates of total energy generated, capital cost, estimated and actual gross income, payback

280 periods and smaller PAT viability for varying turbine scenarios across three PRV sites.

281

282 Acceptable payback periods were identified for those sites with medium and larger power

283 capacities, although the actual payback period was generally higher than the estimated payback across

284 these sites. The installation of a single PAT had the longest payback across all sites whilst the Kaplan

285 was the best turbine choice regarding the shortest payback period. However, the difference in payback

286 between the Kaplan and two PATs was only one year in total. In terms of the PRV with the smallest

287 power potential (Merrion Gates), only the two-PAT scenario was found to have an economically viable

288 payback period. Based on the design flow data (i.e. year 1 only), the initial payback estimates indicated

289 that none of the turbine scenarios would achieve a viable payback period. However, the effects of

290 considerable flow variation over the twenty years meant that the second smaller PAT was the best

291 choice turbine 52% of the time. Figure 5 illustrates the two-PAT scenario in greater detail indicating the

effects of long-term flow variation on turbine efficiency and viability. Evidently, this site exhibits high flow variability and as the flow rate decreases, deviating from the turbine design flow of the larger PAT, the second smaller PAT becomes the better choice in maximising efficiency and power output. Interestingly, the smaller PAT was utilised less frequently across the larger PRV sites due to relatively lower variation in flow conditions.

From an environmental output perspective, Table 3 highlights the total CO_2 emission savings from electricity generation for each turbine option. The Kaplan achieved the greatest savings potential across all PRVs and almost double that of a single PAT at Thomas Court PRV, the site with the largest power output capacity.

Figure 5. Long-term annual flow variation and performance of a two-PAT scenario at Merrion Gates PRV, displaying an annual breakdown of the percentage of time each PAT option was the near-optimal choice in achieving maximum turbine efficiency. PAT 1 represents the larger PAT developed for the design flow whilst PAT 2 is designed for 50% less than the design flow.

Table 3. Comparison of CO_2 emissions savings estimates for varying turbine scenarios across three PRVs in Dublin.

5   Discussion

This research revealed the potential risks posed by long-term flow variation on energy recovery using MHP installations into the future, thus highlighting the importance of this consideration when estimating turbine suitability. The incorporation of high resolution flow and head data allowed for a more realistic assessment of power potential over the long-term given the detailed diurnal, daily, seasonal and annual fluctuations in flow rates which can influence turbine efficiency and viability into the future.

316

5.1 Long-term Flow Variation Across Sites

The analysis of long-term flow and head data across a number of PRVs identified considerable

fluctuations in flow conditions across sites, within the same small geographical region. Thus, site

characteristics such as the district type e.g. commercial or residential, play a strong role in overall

demand requirements. It was anticipated that demand would increase in line with economic and

population growth but not all sites reflected this. The smallest PRV, Merrion Gates, experienced a

reduction in demand during the 2000s. Such variation in flow conditions indicates the complexity in

determining an optimum design flow for a turbine. Thus, anticipating the challenge of long-term flow is

vital when assessing the potential feasibility of varying turbine options. Accordingly, in order to achieve

maximum energy recovery and long-term viability of such installations, improved flexibility in turbine

operation is essential where flow and head are expected to deviate substantially.

5.2 Turbine Comparisons and the Role of PAT Technology in Accommodating Increased Flow
Variability

In order to advance the uptake of MHP technology a viable installation must comprise a minimum

payback period, maximise power output and revenue generation and reduce CO_2 emissions.

Furthermore, it must have the adaptive capacity to accommodate changing flow conditions over the

long-term. The impact of long-term flow and head variation on estimated energy recovery and

investment payback periods across three turbine scenarios revealed some valuable insights.

Firstly, the conventional Kaplan turbine was the best choice in terms of payback periods at the PRV

sites with greater power output potential, whilst a single PAT installation had the longest payback across

all sites. The superior performance of the Kaplan was due to its higher overall efficiency as shown in

339   Figure 3. The Kaplan also maintained higher efficiency over a wider range of partial flows. However,

340   the payback period differed by only one year between the Kaplan and two-PAT scenario. In terms of

341   environmental benefit, the Kaplan produced the greatest reduction in CO_2 emissions; between 37% and

342   48% more than a single PAT and between 25% and 43% more than a two-PAT option when comparing

343   sites. Yet, a significant disadvantage with the Kaplan is that it costs 25% more to install when compared

344   to either a single PAT or two PATs in parallel and this cost differential increased even further when

345   assessing economic viability at the smallest PRV site. Furthermore, the miniaturisation of traditional

346   turbine types such as the Kaplan is known to be prohibitively expensive, rendering them unsuitable for

347   the large number of potential MHP energy recovery sites with small output capacities.

348   The limits of conventional turbines such as the Kaplan are evident at sites with smaller power

349   capacities. The findings indicated that the two-PAT scenario was the only economically viable option at

350   the Merrion Gates PRV site which had the smallest energy recovery potential of 12.5 kW and the

351   greatest flow variability. In contrast the single PAT displayed a significantly longer payback period of

352   16 years whilst the Kaplan had a payback of 11 years and considerable upfront costs. The notably cost

353   effective option of a PAT when compared to a Kaplan, allows for the possibility of integrating more

354   than one PAT in parallel with small additional costs. This turbine solution of multiple PATs with

355   varying design flows in order to cater for flow variation can improve the overall energy generation

356   potential of PATs and the low installation cost coupled with comparable payback periods when

357   compared to conventional turbines highlights its economic advantages.

358   Thus, the integration of PAT technology within water supply networks potentially opens up the

359   opportunity to harness untapped recoverable energy at MHP sites with smaller power generation

360   potential and in locations where there exists large hydraulic variability, sites which may previously have

361   been considered unsuitable for MHP. Indeed it is worth emphasising the importance of this finding

362   where recent research has highlighted that the majority of MHP energy recovery opportunities in water

363 networks in Ireland and the UK were located at PRVs (>67%) and the majority of these sites had small

364 power output capacities (2-20 kW) (Gallagher et al. 2015).

365

366 *5.3 Limitations and Areas for Further Research*

367 The simulation of hypothetical scenarios presented in the current study, where each turbine was

368 designed based on one year of historical data and its performance was assessed across the subsequent 16

369 to 19 years, was useful to examine performance variability over time. However, in practice, hydropower

370 turbine designers will not know the future flow rate or available head over the coming 20 year period,

371 making the design of turbines which cater for future flow variations difficult. The use of water demand

372 forecasting models have an important role to play here to enable the variation in flow at PRVs in water

373 distribution networks to be predicted over the long-term. Corcoran et al. (2016), recently outlined the

374 development of a model of water demand forecasting for MHP installations at PRV sites, where

375 temperature, economic growth, population change, leakage and water pricing were significant

376 influencing factors. This and/or other similar demand forecasting models are a required prerequisite to

377 the design of the two PAT system described here.

378   Furthermore, the current approach presented a two-PAT scenario in which the smaller PAT was

379 designed for 50% less than the design flow. In essence, a range of alternative design flows could be

380 incorporated (80%, 120%, etc.). Optimisation research of various design flow options, in terms of the

381 optimum number and size of PATs, would allow for improved decision making for utility managers

382 regarding the most economically advantageous PAT configuration. Ideally where flow is split between

383 two PATs to cater for flow variation, each PAT should be in operation closer to 50% of the time to

384 achieve a useful benefit from the use of a second turbine. However, such an optimisation requires a

385 prediction of future flow rates at a given site, which may be subject to large uncertainty.

386   A further limitation of the current work lies in the estimation of MHP cost, and particularly PAT

387   cost. PAT costs have been widely reported in literature as being 10-20 times less expensive than

388   conventional turbines and figures in the range of €115-€350/kW have also been published (Teuteberg

389   2010; Motwani et al. 2013; Caravetta et al. 2014b; Power et al. 2014). However a cost-head-power

390   relationship such as that described in Equation 3 for the Kaplan would predict PAT costs with more

391   confidence than the existing cost-power relationship. Further research is required to develop such a

392   relationship for PATs. In addition, future preliminary designs of micro-hydropower energy recovery at

393   PRVs, incorporating analysis of this nature would benefit from the incorporation of a sensitivity

394   analysis. In the absence of the aforementioned PAT cost model, a sensitivity analysis testing the impact

395   of uncertainties such as the cost of PATs, costs of Kaplans and cost of electricity, should be conducted.

396   Assuming that the electromechanical equipment comprise 30% of the total project cost is also

397   subject to error. Gallagher et al. (2015) recently highlighted that in the water network setting this

398   percentage of cost varied from 30% to 70% based on local flow conditions and the size of the

399   installation. However, as the absolute cost of each site is not the valuable contribution of this paper,

400   rather the relative impact of turbine choice at each site and across the sites, this assumption does not

401   adversely impact on the findings.

402

403   **6   Conclusion**

404   Micro-hydropower represents a viable pathway to a more sustainable system of water supply, yet uptake

405   of this technology remains low and sporadic due to a range of risk factors which include long-term flow

406   and head variations potentially impacting on economic viability assessments. The focus of this study

407   was to undertake a detailed investigation of the impact of long-term flow variability on turbine operating

408   efficiencies and power output across a number of turbine design scenarios over a twenty year period

409   using Dublin as a case study site. Findings revealed that considerable variation in long-term flow

conditions occurred over the 20 years, particularly at PRV sites with smaller power generation capacities, while head levels did not vary to the same degree.

Following investment payback analysis, the Kaplan was found to have the shortest payback period and achieved the largest saving in CO_2 emissions across both medium and large MHP sites. However, neither the conventional turbine nor single PAT were found to be economically viable, at the most commonly occurring, smallest PRV site. Although there was an evident reduction in power generation, the two-PAT scenario proved to be economically viable despite the increased flow variability. Furthermore, this option was almost comparable with the Kaplan in terms of payback period across the remaining sites and had a significantly lower installation cost.

Therefore, the incorporation of multiple PATs in parallel represents a viable technology option which demonstrates resilience and flexibility to future fluctuations in flow and head conditions, enhancing the adaptive capacity of MHP systems into the long-term. Previous investigations examining the available resources for MHP energy recovery have highlighted that the majority of potential MHP sites lies in this small capacity range, similar to the Merrion Gates site examined here. Such sites would not have been previously considered economically viable due to the extent of flow variation and low power output.

The present study is of relevance for water utilities as it highlights an adaptive design option to maximise energy recovery potential within water distribution networks. Accordingly, the findings strengthen the evidence base for greater uptake of MHP technology and PATs.

434

References

436 Agarwal, T. (2012). Review of pump as turbine (PAT) for micro-hydropower. *Int. J. Emerg. Tech. &*

437   *Adv. Eng.,* 2, 163–169.

438 Carravetta, A., Del Giudice, G., Fecarotta, O., and Ramos, H.M (2013). PAT design strategy for energy

439   recovery in water disribution networks by electrical regulation. *Energies*., 6, 411–424.

440 Carravetta, A., Del Giudice, G., Fecarotta, O., and Ramos, H.M. (2012). Energy Production in Water

441   Distribution Networks: A PAT Design Strategy. *Water Resour. Manage.,* doi: 10.1007/s11269-012-

442   0114-1

443 Carravetta, A., Fecarotta, O., Del Giudice, G., and Ramos H.M. (2014a). Energy recovery in water

444   systems by PATs: A comparisons among the different installation schemes. *Proc. Eng.,* 70, 275–

445   284. doi: 10.1016/j.proeng.2014.02.031

446 Carravetta, A., Fecarotta, O., Sinagra, M., and Tucciarelli, T. (2014b). Cost Benefit Analysis for

447   Hydropower Production in Water Distribution Networks by a Pump as Turbine. *J. Water. Resour.*

448   *Plann. Manage*. doi: 10.1061/(ASCE)WR.1943-5452.0000384

449 Colombo, A., and Kleiner, Y. (2011). Energy recovery in water distribution systems using

450   microturbines. *Prob. Method. Water Waste. Eng.* 23-27, 1–9.

451 Corcoran, L., Coughlan, P., and McNabola, A. (2013). Energy Recovery Potential using Micro

452   Hydropower in Water Supply Networks in the UK & Ireland. *Water Sci. & Tech. Water Sup.* 13,

453   552–560.

454 Corcoran, L., McNabola, A., and Coughlan, P. (2012). Energy recovery potential of the Dublin region

455   water supply network. Proc., *Int. Water Association: World Congress on Water, Climate and*

456   *Energy*. IWA Publishing, Dublin, Ireland.

457   Corcoran, L., McNabola, A., and Coughlan, P. (2016). Predicting and quantifying the effect of variations

458   in long-term water demand on micro-hydropower energy recovery in water supply networks. *Urb.*

459   *Water*. In Press

460   Daigger, G.T. (2009). Evolving urban water and residuals management paradigms: Water reclamation

461   and reuse, decentralization and resource recovery. *Water Env. Res.* 81, 809-823

462   DEFRA (2008). *Future water- The Government's water strategy for England*. Department for

463   Environment, Food and Rural Affairs. Norwich

464   DoEHLG (2010). *Report of the Local Government Efficiency Review Group*. Department of the

465   Environment, Heritage and Local Government. Dublin

466   Environment Agency (2009). *Renewable energy potential for the water industry.* Bristol

467   ESHA (2004). *Guide on How to Develop a Small Hydropower Plant*. European Small Hydropower

468   Association. Brussels

469   Eurostat (2014a). *Hydropower*. In: Energy Resources - Hydropower.

470   http://epp.eurostat.ec.europa.eu/portal/page/portal/data_centre_natural_resources/natural_resources/

471   energy_resources/hydropower. Accessed 1 Dec 2015

472   Eurostat (2014b). *Electricity and natural gas price statistics*. In: Eurostat Statistics Explained

473   http://epp.eurostatec.europa.eu/statisticsexplained/index.php/. Accessed 1 Dec 2015

474   Fecarotta, O., Aricò, C., Carravetta, A., Martino, R., and Ramos, H.M. (2015). Hydropower Potential in

475   Water Distribution Networks: Pressure Control by PATs. *Water Resourc. Manage.* 29, 699–714.

476   doi: 10.1007/s11269-014-0836-3

477   Gaius-obaseki T (2010). Hydropower opportunities in the water industry. *International Journal of*

478   *Environ. Sci.* 1, 392–402.

479   Gallagher, J., Harris, I.M., Packwood, A.J, McNabola, A., and Williams, A.P. (2015). A strategic

480   assessment of micro-hydropower energy recovery in the UK and Irish water industry: Identifying

481   technical and economic constraints. *Renew. Energy*. doi: 10.1016/j.renene.2015.03.078

482 García, J.P., Marco, A.C., and Santos, S.N. (2010). Use of centrifugal pumps operating as turbines for

483   energy recovery in water distribution networks. Two case study. *Adv. Materials Res.* 107, 87–92.

484 Giugni, M., Fontana, N., and Ranucci, A. (2014). Optimal Location of PRVs and Turbines in Water

485   Distribution Systems. *J. Water Resourc. Plann. Manag.* doi: 10.1061/(ASCE)WR.1943-

486   5452.0000418.

487 Koch, H., Vögele, S., Kaltofen, M., Grossmann, M., and Gunewald, U. (2014). Security of Water

488   Supply and Electricity Production: Aspects of Integrated Management. *Water Resourc. Manage.*

489   28, 1767–1780. doi: 10.1007/s11269-014-0589-z

490 Kwok, S.C., Lang, H., and O'Callaghan, P. (2010). *Water Technology Markets 2010: Key Opportunities*

491   *and Emerging Trends*. Oxford

492 McNabola, A., Coughlan, P., Corcoran, L., Power, C., Williams, A.P., Harris, I.M., Gallagher, J., and

493   Styles, D. (2014a). Energy recovery in the water industry using micro-hydropower: An opportunity

494   to improve sustainability. *Water Pol.* 16, 168–183.

495 McNabola, A., Coughlan, P. and Williams, A.P. (2014b). Energy recovery in the water industry: An

496   assessment of the potential of micro-hydropower. *Water Environ. J.* 28, 294–304.

497 Motwani, K.H., Jain, S.V., and Patel, R.N. (2013). Cost Analysis of Pump as Turbine for Pico

498   Hydropower Plants – A Case Study. *Proc. Eng.* 51, 721-726.

499 Nautiyal, H. and Varun, K.A. (2010). CFD analysis on pumps working as turbines. *Hydro Nepal* 6, 35–

500   37.

501 Ogayar, B., Vidal, P.G. and Hernandez, J.C. (2009). Analysis of the cost for the refurbishment of small

502   hydropower plants. *Renew. Energy,* 34, 2501–2509. doi: 10.1016/j.renene.2009.03.027

503 Ørke, P.R. (2010). Zeropex: Pressure control with a payback. *All Energy Exhibition and Conference*

504   *2010*.

505 Paish, O. (2002). Small hydro power: Technology and current status. *Renew. Sust. Energy Rev.* 6, 537–

506   556.

507 Power, C., Coughlan, P., and McNabola, A. (2014). Development of an evaluation method for

508   hydropower energy recovery in wastewater treatment plants: Case studies in Ireland and the UK.

509   *Sust. Energy Tech. Assess.* 7, 166–177.

510 Ramos, H. and Borga, A. (1999). Pumps as turbines: An unconventional solution to energy production.

511   *Urb. Water* 1, 261–263. doi: 10.1016/S1462-0758(00)00016-9

512 Saket, R.K. (2008). Design, development and reliability evaluation of a micro hydro-power generation

513   system based on municipal waste water. *Proc. IEE Electrical Power and Energy Conference.*

514   Vancouver, pp1–8

515 Samora, I., Hasmatuchi, V., Münch-Alligné, C., Franca, M.J., Schleiss, A.J., and Ramos, H.M. (2016).

516 Experimental characterization of a five blade tubular propeller turbine for pipe inline installation.

517 *Renew. Energy*. 95, 356-366.

518 SEAI, (2013). *Energy in Ireland: Key Statistics 2013*. Dublin

519 Sitzenfrei, R. and Rauch, W. (2015) Optimizing Small Hydropower Systems in Water Distribution

520   Systems Based on Long-Time-Series Simulation and Future Scenarios. *J. Water Resourc. Plann.*

521   *Manage.* doi: 10.1061/(ASCE)WR.1943-5452.0000537.

522 Sitzenfrei, R. and Von Leon, J. (2014). Long-time simulation of water distribution systems for the

523   design of small hydropower systems. *Renew. Energy* 72, doi: 10.1016/j.renene.2014.07.013

524 Sitzenfrei, R., Von Leon, J. and Rauch, W. (2014). Design and optimization of small hydropower

525   systems in water distribution networks based on 10-years simulation with Epanet2. *Proc. Eng. 89,*

526   533–539.

527 Sweeney, J., Albanito, F., Brereton A, et al. (2008). *Climate Change – Refining the Impacts for Ireland*.

528   EPA STRIVE Report. Wexford Ireland.

529 Teuteberg, B. (2010). *Design of a Pump Microhydro System for an Abalone Farm*. Stellenbosch

530   University.

531   UKWIR, (2010). *Energy Efficiency in the Water Industry: A Compendium of Best Practices and Case*

532   *Studies - Global Report*. London

533   Vicente, D.J., Garrote, L., Sánchez, R., and Santillán, D. (2016) Pressure Management in Water

534   Distribution Systems: Current Status, Proposals, and Future Trends. *J. Water Resour. Plann.*

535   *Manage*. 142(2): 04015061

536   Vilanova, M.R.N. and Balestieri, J.A.P. (2014). Hydropower recovery in water supply systems: Models

537   and case study. *Energy Con. Manage.* 84, 414–426. doi: 10.1016/j.enconman.2014.04.057

538   Williams, A.A., Smith, N., Bird, C. and Howard, M. (1998). Pumps as turbines and induction motors as

539   generators for energy recovery in water supply systems. *J. Chart. Inst. Water Environ. Manage.*

540   12,175–178.

541   Williams, A.A. (1996). Pumps as turbines for low cost micro hydro power. *Proc. World Renew. Energy*

542   *Congress*. 1227–1234

543   Zilberman, D., Sproul, T., Rajagopal, D., Sexton, S., and Hellegers, P. (2008). Rising energy prices and

544   the economics of water in agriculture. *Water Pol.* 10,11–21.

545

Table 1**.** An overview of flow, head and power output estimates for three PRVs in Dublin. Power estimates were based on varying Kaplan turbine efficiencies, where the turbine design flow/pressure was assumed to be the average of the data from year 1 of the record.
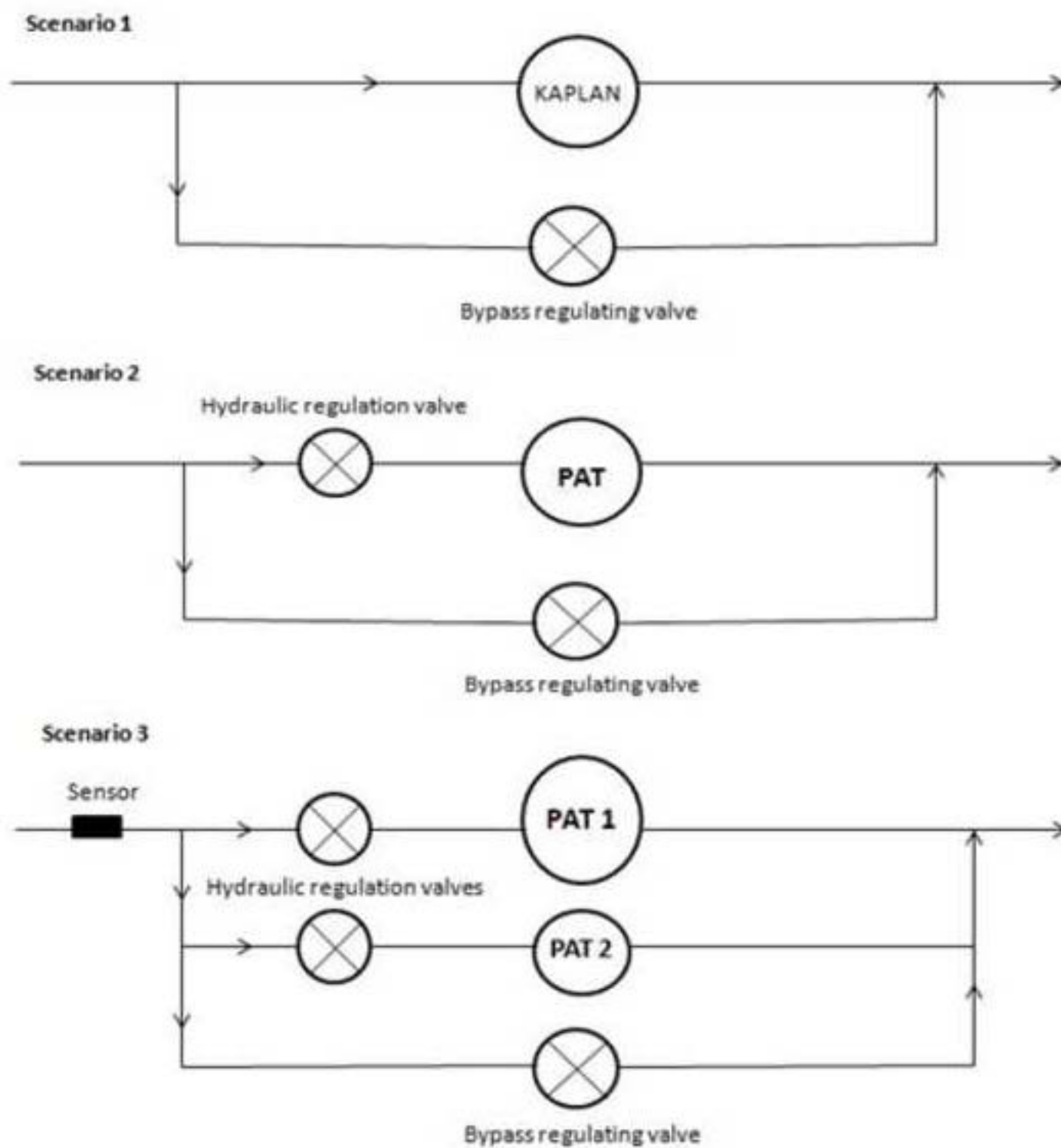
PRV Location	Flow (m^3/s)	Head (m)	Estimated Power Output (kW)
Thomas Court	0.18	70.97	145.15
Blackhorse Bridge	0.24	43.9	75.44
Merrion Gates	0.32	7.84	12.5

Table 2   Click here to download Table Table 2 Rev A.docx ±

Table 2. Estimates of total energy generated, capital cost, estimated and actual gross income, payback periods and smaller PAT viability for varying turbine scenarios across three PRV sites.

PRV Site	Turbine Scenario	Capital cost (€)	Estimated gross income (€/yr)	Estimated payback (Years)	Total Generation (kWh/yr)	Actual gross income (€/yr)	Actual payback (Years)	% of time smaller size PAT in use
Thomas	Kaplan	509,080	153,656	3	943,520	127,766	4	
Court	PAT	376,200	59,109	6	487,692	65,318	7	
(145 kW)	2 PATs	386,121	67,308	6	542,337	72,804	5	35
Blackhorse	Kaplan	409,392	83,272	5	875,574	118,458	5	
Bridge	PAT	306,921	63,830	5	554,964	74,534	6	
(75 kW)	2 PATs	317,095	66,927	5	586,298	80,323	6	26
Merrion	Kaplan	235,375	12,621	19	185,440	23,909	11	
Gates	PAT	168,129	9,387	18	103,879	12,735	16	
(12.5 kW)	2 PATs	169,813	10,164	17	139,003	17,547	10	52

Table 3

Table 3. Comparison of CO_2 emissions savings estimates for varying turbine scenarios across three PRVs in Dublin.

PRV Site	Turbine Scenario	Total CO_2 emissions savings (tonnes)
Thomas Court (145 kW)	Kaplan	8967
	PAT	4635
	2 PATs	5154
Blackhorse Bridge (75 kW)	Kaplan	7396
	PAT	4688
	2 PATs	4953
Merrion Gates (12.5 kW)	Kaplan	1860
	PAT	1042
	2 PATs	1394

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Scenario 1 — KAPLAN, Bypass regulating valve

Scenario 2 — Hydraulic regulation valve, PAT, Bypass regulating valve

Scenario 3 — Sensor, Hydraulic regulation valves, PAT 1, PAT 2, Bypass regulating valve

Figure 2

Dublin

Thomas Court (145 kW)

Blackhorse Bridge (75 kW)

Merrion Gates (12.5 kW)

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Figure 3

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Figure 3

Figure 4

a)

b)

c)
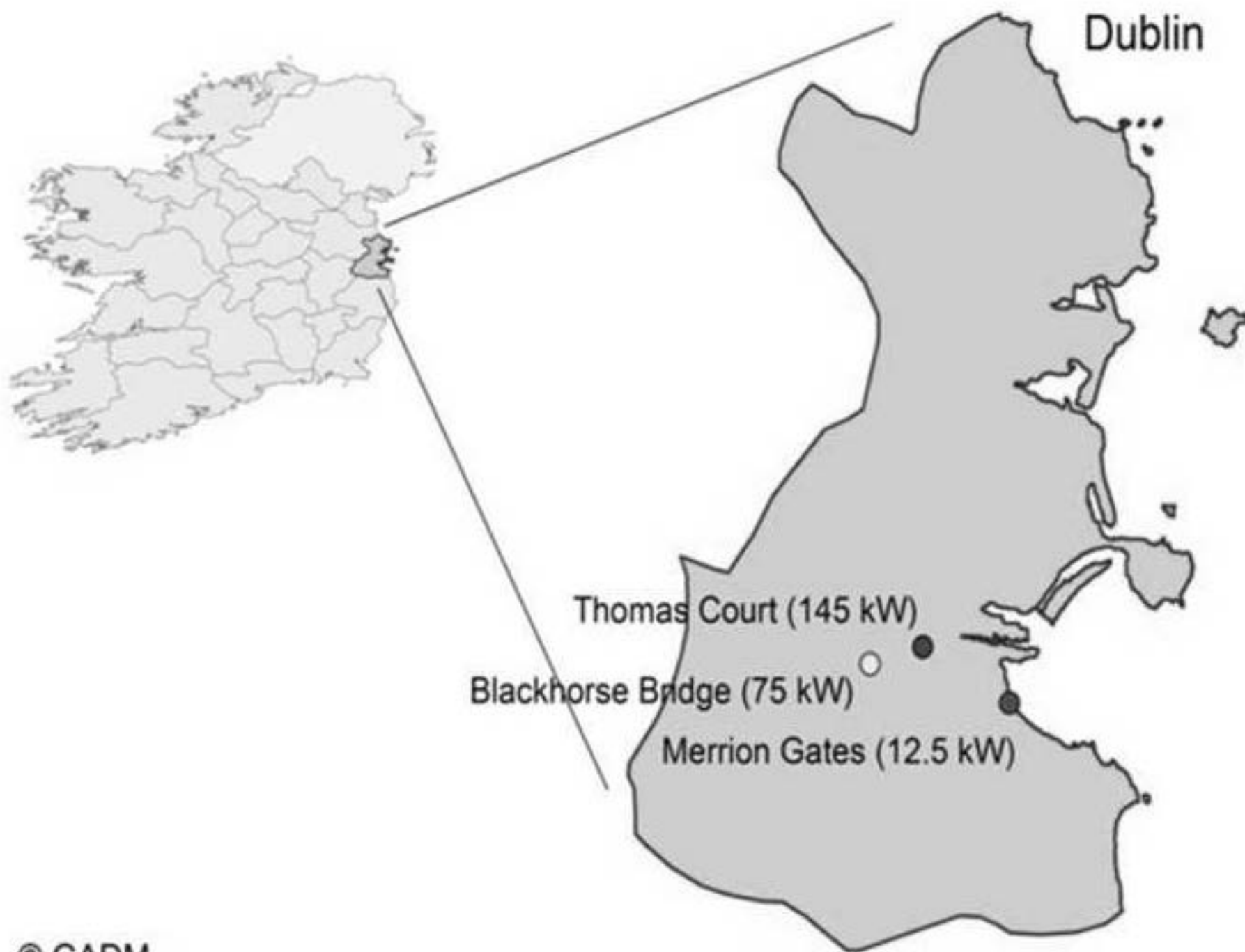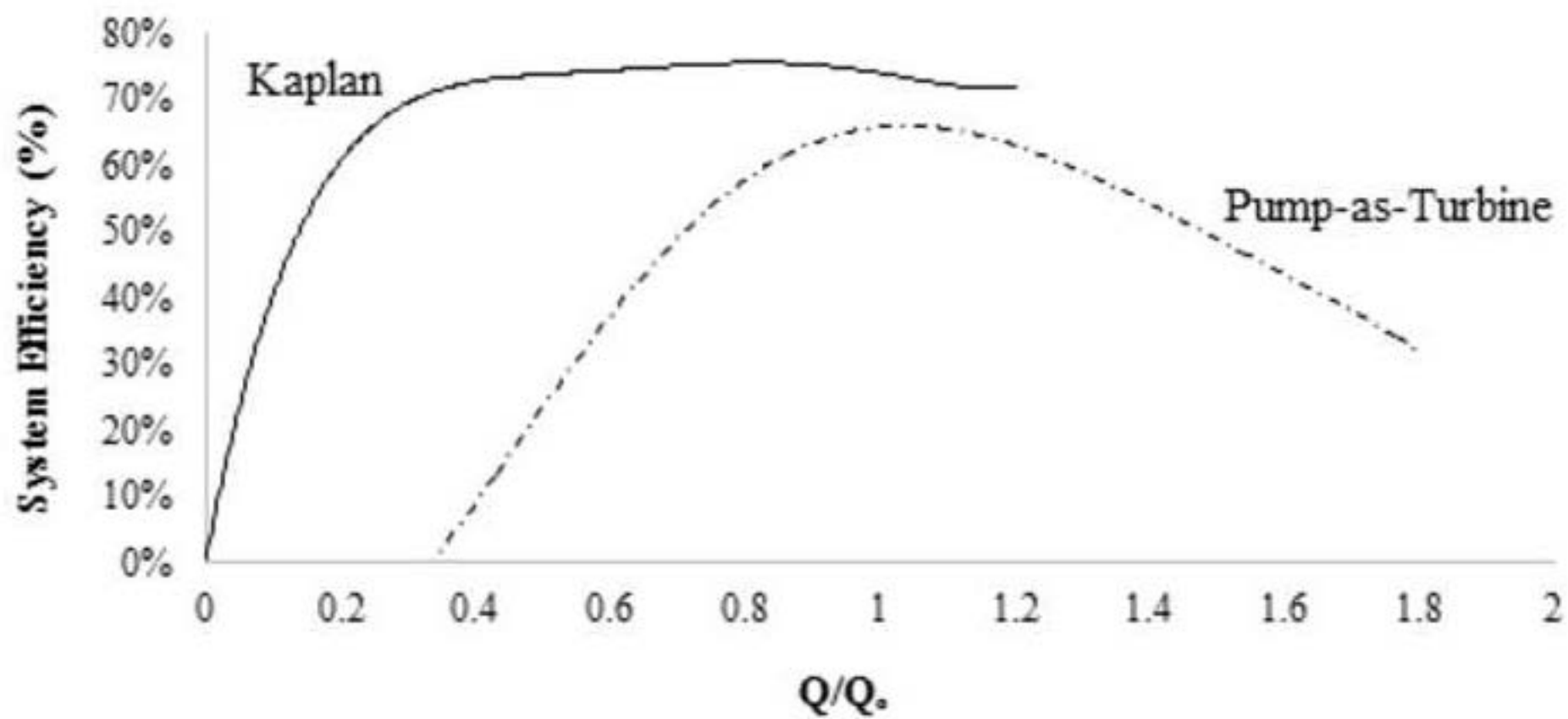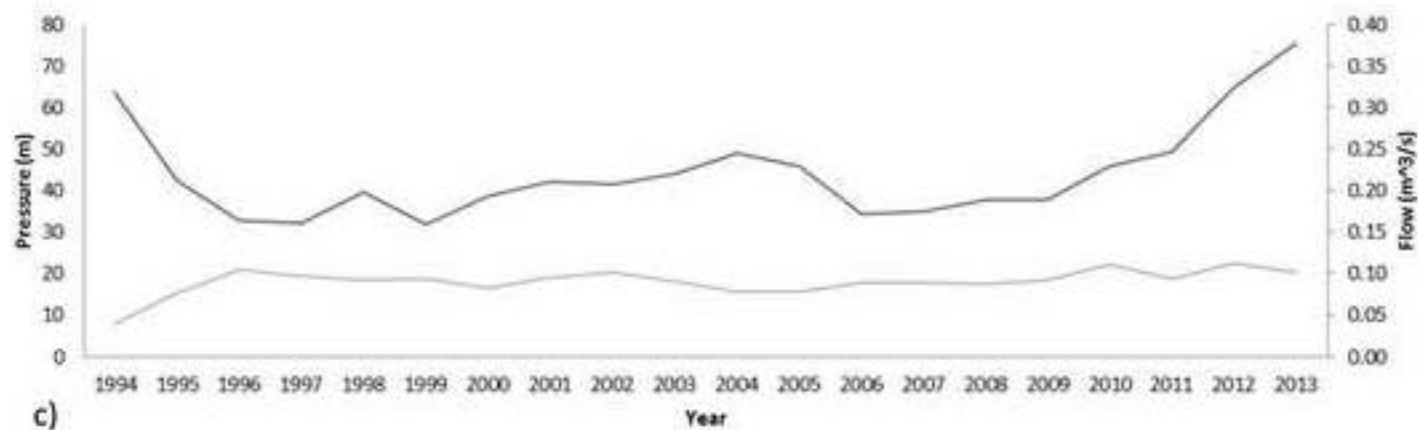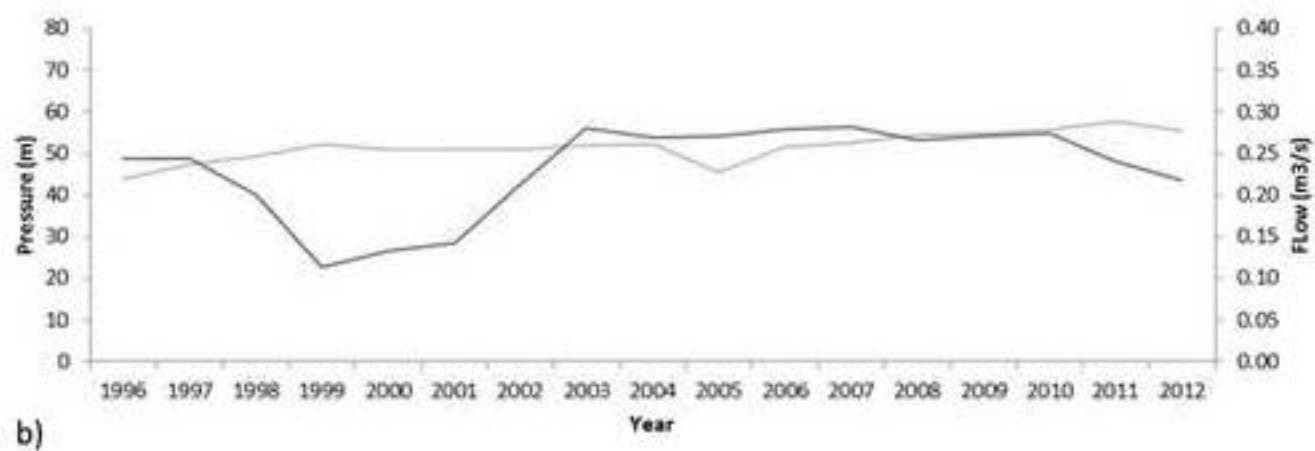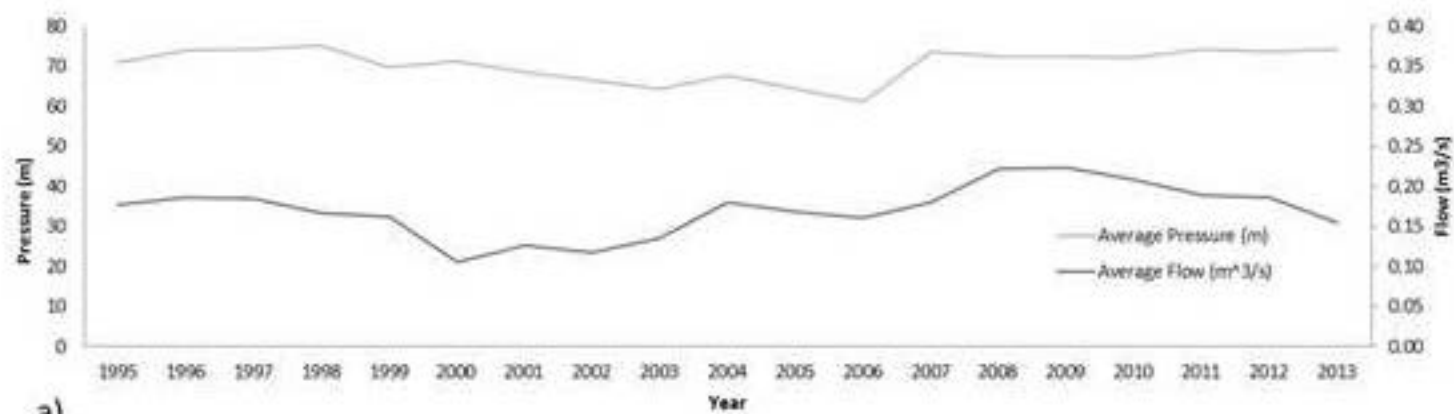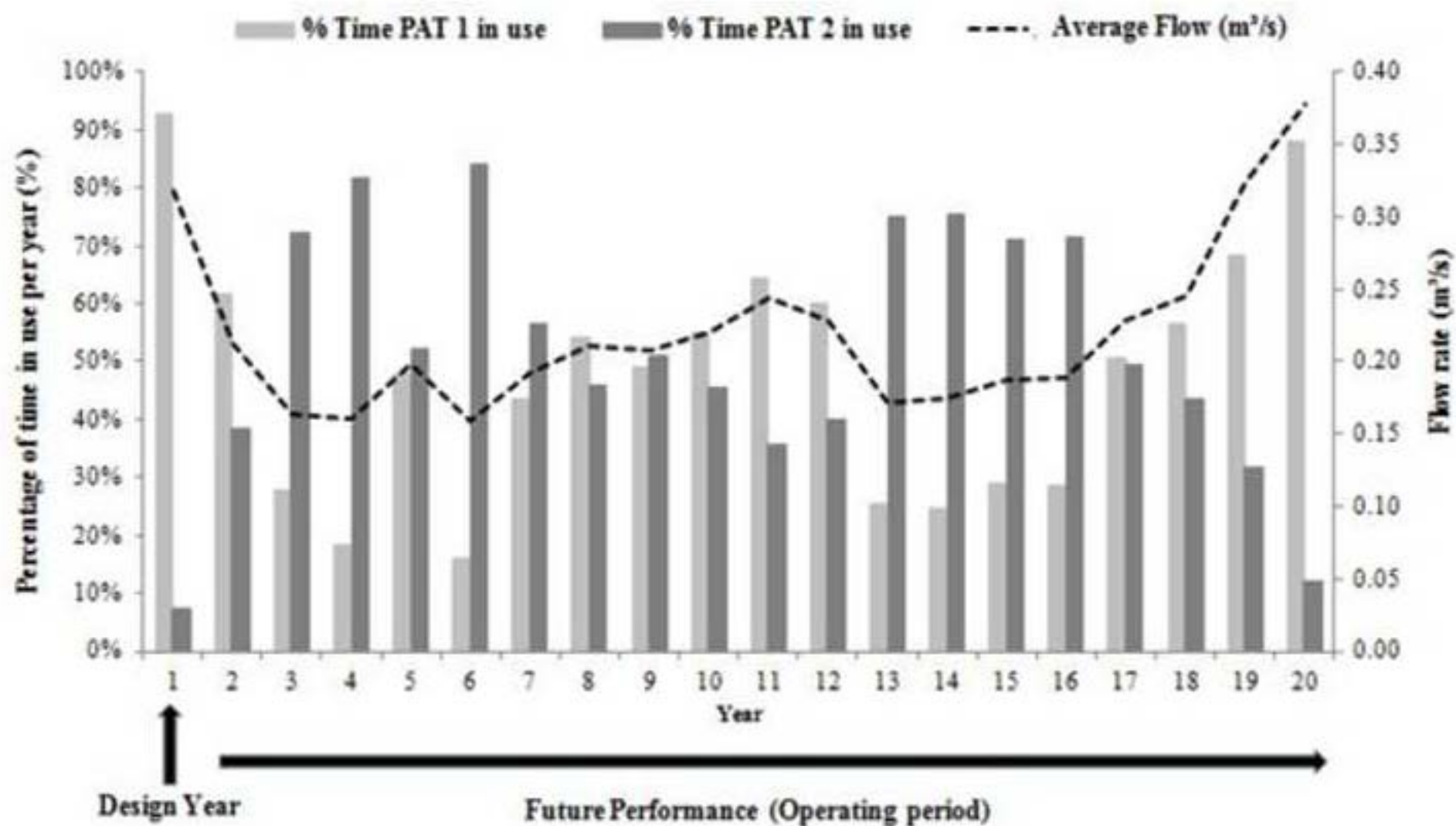
Figure 1. Installation schemes of three turbine scenarios; a Kaplan turbine, PAT and two PATs in parallel (Adapted from Carravetta *et al.* (2012)).

Figure 2. Map of the Dublin region displaying the location of the three pressure reducing valves used in the case study.

Figure 3. Overall system efficiency curves for the Kaplan turbine and PAT, assuming generator and transmission loss efficiencies of 85% and 98% respectively.

Figure 4. Long-term flow and pressure variation across three PRV sites. a) Thomas Court, 145 kW (1994 - 2013); b) Blackhorse Bridge, 75 kW (1996 - 2013) and c) Merrion Gates, 12.5 kW (1993 – 2013).

Figure 5. Long-term annual flow variation and performance of a two-PAT scenario at Merrion Gates PRV, displaying an annual breakdown of the percentage of time each PAT option was the near-optimal choice in achieving maximum turbine efficiency. PAT 1 represents the larger PAT developed for the design flow whilst PAT 2 is designed for 50% less than the design flow.

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