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Streamwise turbulence modulation in non-uniform open-channel clay suspension

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2 M.G.W. de Vet¹, R. Fernández^{1,2}, J.H. Baas³, W.D. McCaffrey⁴ and R. M. Dorrell¹ 3 ¹ Energy and Environment Institute, University of Hull. Hull HU6 7RX, U.K. 4 ² Department of Civil and Environmental Engineering, Penn State University. University Park, 5 PA 16802, USA. 6 ³ School of Ocean Sciences, Bangor University, Menai Bridge, Anglesey LL59 5AB, U.K. 7 ⁴ School of Earth and Environment, University of Leeds, Leeds LS2 9 JT, U.K. 8 9 Corresponding author: Marijke de Vet (m.de-vet@hull.ac.uk) 10 **Key Points:** 11 Comparable to uniform flow, the combination of flow velocity and clay concentration 12 influences the clay flow type in non-uniform flows 13 Accelerating clay-laden flows adapt faster to velocity changes than decelerating flows; 14 15 breaking clay bonds is easier than establishing them Adaptation timescales grow with clay concentration for decelerating clay-laden flows 16 passing through a larger variety of clay flow types 17

Abstract

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- 21 Cohesive sediment particles are ubiquitous in environmental flows. The cohesive properties of
- clay promote the formation of clay flocs and gels and relatively small suspended clay
- 23 concentrations can enhance or suppress turbulence in a flow. Furthermore, flows are naturally
- 24 non-uniform, varying in space and time, yet the dynamics of non-uniform open-channel clay
- suspension flows is poorly understood. For the first time, the adaptation time and length scales of
- 26 non-uniform clay suspension flows were quantified using novel experiments with spatially
- varying, but temporally uniform flow. Different levels of turbulence enhancement and
- attenuation were identified as the flow decelerates or accelerates. Results highlight that
- 29 decelerating clay suspension flows crucially have a longer adaptation time than accelerating clay
- 30 suspension flows. This is explained by the longer timescale required for formation of bonds
- 31 between cohesive particles in turbulence attenuated flows after deceleration than the rapid
- breakdown of bonds in turbulent flows after acceleration of clay suspension flows. This
- 33 hysteresis is more pronounced for higher concentration decelerating flows that pass through a
- larger variety of clay flow types of turbulence enhancement and attenuation. These different
- 35 adaptation time scales and associated clay flow type transitions are likely to affect clay flow
- 36 dynamics in a variety of fluvial and submarine settings.

Plain Language Summary

- Flows in natural environments, such as rivers, estuaries, seas, and oceans, can transport sediment
- in suspension. The suspended sediment can increase or decrease turbulence in a flow, depending
- on the sediment concentration. Clay has the ability to form bonds between the individual
- 41 particles and therefore even small concentrations are sufficient to alter turbulence levels in a
- flow. The amount of alteration of turbulence is known for uniform, constant flow conditions, but
- in natural environments, flows are often non-uniform. For example, flow variations can occur
- due to changes in river width or bed slope. The influence of these variations on clay suspension
- 45 flows is unknown. New physical experiments were conducted where clay suspension flows were
- decelerated and accelerated. As the flow decelerates, turbulence in the flow is reduced and bonds
- between the suspended clay particles are established. Turbulence increases as the flow
- 48 accelerates and clay bonds are broken. Decelerating flow requires more time to adjust to changes
- in velocity than accelerating flow, as establishing the bonds between clay particles requires more
- 50 time than breaking them. This means that, especially for the decelerating flows, the influence of
- a change in velocity is noticeable further downstream.

1 Introduction

- 53 Cohesive sediment-laden flows are important in a wide range of natural environments, such as
- rivers, estuaries, shallow seas and deep oceans (Whitehouse et al., 2000; Winterwerp and van
- Kesteren, 2004), and in industrial settings (Ackers et al., 2001). For example, cohesive sediment
- supply to rivers can be increased by high-magnitude, low-frequency events, such as storms,
- floods and post-wildfire erosion (Swanson, 1981; Sankey et al., 2017), which occur more often
- because of climate change (Geertsema et al., 2006; Reneau et al., 2007; Barbero et al., 2015).
- 59 Furthermore, cohesive sediment is common in submarine gravity currents, such as turbidity
- currents, hybrid events, mass flows and associated deposits (Talling et al., 2012). The increases
- in sediment transport can have major impacts on water quality and aquatic ecosystems, including
- fish habitats, and channel morphology (Smith et al., 2011). High suspended cohesive sediment

concentrations modify flow dynamics by either enhancing (Best et al., 1997; Baas and Best, 2002) or dampening turbulence (Bagnold, 1954; Wang and Larsen, 1994), influencing sediment transport rates and erosion and deposition patterns (Partheniades, 1965; Metha et al., 1989).

Cohesive clay particles may collide and form larger particles, or flocs, when the distance between the particles is sufficiently small (Van Olphen, 1977; Winterwerp and van Kesteren, 2004). Networks of flocs in the flow, i.e., clay gels, enhance viscosity and yield stress, and thus are a key control on flow turbulence (Baas and Best, 2002). Research into steady, uniform clay flows indicate a close interaction between turbulent and cohesive forces, controlling the dynamic structure of clay flows (Baas and Best, 2002; Baas et al., 2009). As the clay concentration increases, it becomes increasingly difficult to break the cohesive bonds between particles, resulting in the formation of a pervasive network of permanently interlinked clay particles; turbulent energy is dissipated by the high effective viscosity, and the flow becomes laminar. Conversely, the electrostatic bonds between the clay particles can be broken in regions of high shear. Thus, an increase in turbulence generation in the flows by, for example, an increasing flow velocity has the potential to break bonds between the clay particles and reduce the flow viscosity (Partheniades, 2009). This shifting balance between turbulent and cohesive forces regulates the dynamic structure of cohesive flows (Baas et al., 2009).

Baas et al. (2009) defined a clay flow classification scheme based on flume experiments. The only technique available for velocity measurements in high concentrated flows is Ultrasonic Velocity Profilers, which are designed to work along a single beam. This allows velocity measurements to be collected in one flow direction and consequently, Baas et al. (2009) based the clay flow classification scheme on streamwise velocity measurements instead of a 3D turbulence field. The clay flow classification scheme consists of five different clay flow types in order of increasing clay concentration: turbulent flow, turbulence-enhanced transitional flow, lower transitional plug flow, upper transitional plug flow, and quasi-laminar plug flow (Fig. 1). Turbulent flow exhibits a logarithmic velocity profile with an associated decrease in turbulence intensity away from the bed (Nezu and Nakagawa, 1993). The velocity of turbulence-enhanced transitional flows progressively diminishes, in particular close to the base of the flow, accompanied by a progressive increase in turbulence intensity over the full flow depth, whilst the logarithmic velocity profile is maintained. A progressive increase in clay concentration in lower transitional plug flows results in the formation of a plug, which thickens from the water surface downwards. This flow type exhibits a decreased near-bed velocity and increased near-bed turbulence in combination with decreased turbulence intensity in the outer flow. The plug flow further thickens downwards in upper transitional plug flows with increasing clay concentration, whilst the maximum turbulence intensity moves away from the bed and decreases. The upward shift in turbulence production is explained through thickening of the viscous sublayer (Best and Leeder, 1993; Li and Gust, 2000) and the development of an internal shear layer (Baas and Best, 2002), which separates the near-bed region from the plug flow region. Further increasing the clay concentration results in fully suppressed turbulence in quasi-laminar plug flows, apart from minor residual turbulence near the base of the flow in a thin shear layer.

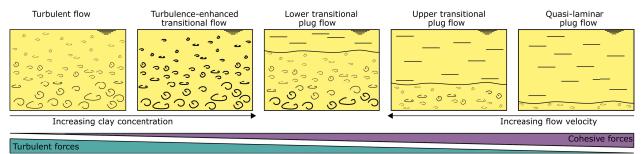


Figure 1. Schematic model of the balance between cohesive and turbulent forces that determines the behaviour of turbulent, transitional, and laminar clay-laden flows, divided into five different clay flow types after the classification scheme of Baas et al. (2009). Modified after Baas et al. (2009).

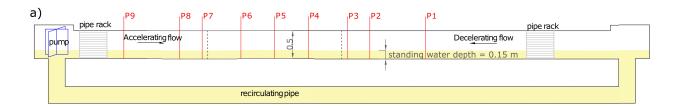
Flows are naturally non-uniform; here, flow non-uniformity is taken to refer to streamwise changes in depth-averaged velocity. The effect of clay on streamwise decelerating and accelerating flow is essential for understanding sediment-laden flow dynamics. The formation of bonds between cohesive sediment particles is a time-dependent (thixotropic) process and, therefore, cohesive-sediment laden flows need time to adjust to spatial variations in flow velocity. However, the changing balance between turbulent and cohesive forces in clay-laden flows under non-uniform conditions is poorly understood. Understanding this balance is pivotal, as erosion, transport, and deposition of sediment depend on the magnitude and distribution of flow turbulence (Dorrell et al., 2018). Spatio-temporal increases and decreases in turbulence directly affect the transport capacity and deposition and erosion patterns (Dorrell and Hogg, 2012; Moody et al., 2013).

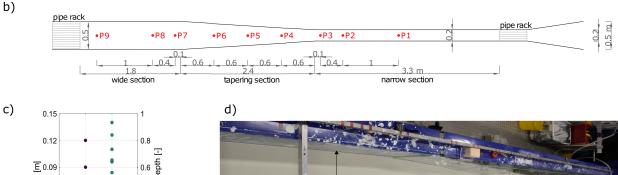
An increased understanding of the influence of cohesive sediment on non-uniform flow conditions is needed. This paper details experimental results on the flow structure of clay-laden flows, for the first time isolating the effect of non-uniformity by spatial deceleration and acceleration in open-channel flows. The aim is to understand the adaptation of clay-laden flows to non-uniform flow conditions. We address the following research questions: (1) What are the mean flow and streamwise turbulence characteristics of horizontally decelerating and accelerating clay-laden flows (Section 3)? (2) How do non-uniform flows with different suspended clay concentration compare to each other and to uniform clay-laden flows, i.e. which clay flow types can be identified in clay-laden decelerating and accelerating flows (Section 4.1)? (3) How much time do decelerating and accelerating flows need to adapt to the changing flow conditions (Section 4.2)? (4) Are there differences in adaptation between decelerating and accelerating clay-laden flows (Section 4.3)?

2 Methodology

Mixtures of pure kaolinite (Imerys Polwhite-E, median particle size $D_{50} = 9 \mu m$, sediment density $\rho_s = 2600 \text{ kg m}^{-3}$) and fresh water were circulated through a horizontal hydraulic flume by means of a variable-discharge slurry pump (Fig. 2a). The flume was 10 m long and 0.5 m wide, with a standing water depth, h_0 , of 0.15 m. At the upstream end, the flume contained a turbulence-damping grid to straighten the flow. The flow moved over a flat, smooth floor downstream of the turbulence-damping grid. An inset channel was placed in the flume. It had a 0.2 m wide narrow section and a 2.4 m long tapering section. This division in the flume results in a flume expansion or narrowing with a ratio of 1 to 16; this smooth transition avoided flow

separation or recirculation cells. The inset forced the flow through a narrow to wide transition (decelerating flows) or through a wide to narrow transition (accelerating flows) depending on the flow direction (Fig. 2b). Thus, in contrast to earlier work in non-tapering flumes (Baas and Best, 2002; Baas et al., 2009), this channel design enabled controlled spatial changes in the flow velocity and turbulence to be measured.





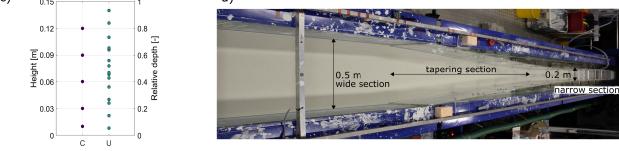


Figure 2. a) Side view of the experimental setup, b) top view of the inset channel, with points P indicating measurement locations, c) velocity (U) and sediment concentration (C) measurement positions above the channel bed; relative depth = height / depth, d) photo of the flume setup. All dimensions in meters.

2.1 Experimental conditions

Table 1 shows the range of clay concentrations and flow velocities used; control experiments were conducted with clear water. Clay was soaked in water for a minimum of one day before adding the clay suspension to the flume, to guarantee that no dry clumps remained. To ensure a uniform mixture of clay and water in the flume, initially, the flume was run at high rotational speed of the slurry pump for 30 minutes combined with additional mixing in the wide section using a hand-held mixture. Afterwards, the flume ran for 16 to 20 hours to allow the clay-laden flows to reach equilibrium conditions and allow for any deposition of clay before measurements were taken. This allowed assessment of streamwise turbulence dynamics of non-uniform clay-laden flows without influence of erosional or depositional processes. Control measurements of the velocity were collected 3 hours after experimental runs to confirm the establishment of equilibrium conditions.

Table 1. Experimental conditions at selected positions in the flume. Q = discharge, based on velocity measurements at P2 with assumed minimal change in velocity over the flume width; C = spatial-averaged volumetric concentration, based on an average of suspended sediment samples over the depth and along the length of the flume; $h_0 =$ standing water depth at P8; T = water temperature; $\overline{U} =$ depth-averaged velocity; Fr = Froude number; Re = Reynolds number. The labelling of experimental runs is defined using D for decelerating and A for accelerating flows and the value of clay concentration.

| Experimental | Q | С | h_0 | T | Measuring | $\overline{\overline{U}}$ | Fr | Re |
|-------------------|-----------|---------------|--------|----------|-----------|---------------------------|------|-----------------|
| run | | | | | point | | | |
| | $[m^3/s]$ | [vol %] | [m] | [°C] | | [m/s] | [-] | $[-\cdot 10^4]$ |
| Decelerating flow | | | | | | | | |
| D1-C0.0 | 0.021 | 0.00 | 0.150 | 16.0 | P2 | 0.69 | 0.57 | 10.3 |
| | | | | | P5 | 0.52 | 0.43 | 7.8 |
| | | | | | P8 | 0.33 | 0.27 | 4.9 |
| D2-C0.0 | 0.015 | 0.00 | 0.158 | 17.6 | P2 | 0.49 | 0.40 | 7.8 |
| | | | | | P5 | 0.38 | 0.30 | 5.9 |
| | | | | | P8 | 0.28 | 0.23 | 4.5 |
| D3-C0.9 | 0.014 | 0.92 | 0.150 | 18.7 | P2 | 0.48 | 0.39 | 6.7 |
| | | | | | P5 | 0.35 | 0.29 | 4.9 |
| | | | | | P8* | 0.28 | 0.23 | 4.0 |
| D4-C1.5 | 0.019 | 1.47 | 0.150 | 18.0 | P2 | 0.64 | 0.53 | 8.3 |
| | | | | | P5 | 0.45 | 0.37 | 6.0 |
| | | | | | P8 | 0.33 | 0.27 | 4.3 |
| D5-C2.7 | 0.016 | 2.67 | 0.150 | 18.0 | P2 | 0.54 | 0.45 | 5.8 |
| | | | | | P5 | 0.42 | 0.35 | 4.5 |
| | | | | | P8 | 0.27 | 0.22 | 2.9 |
| | | | Ассе | lerating | flow | | | |
| A1-C0.0 | 0.015 | 0.00 | 0.170 | 17.6 | P2 | 0.45 | 0.35 | 7.6 |
| | | | | | P5 | 0.26 | 0.20 | 4.4 |
| | | | | | P8 | 0.16 | 0.13 | 2.7 |
| A2-C1.4 | 0.014 | 1.39 | 0.170 | 18.0 | P2 | 0.41 | 0.32 | 6.2 |
| | | | | | P5 | 0.26 | 0.20 | 3.9 |
| | | | | | P8* | 0.20 | 0.17 | 3.0 |
| A3-C1.5 | 0.016 | 1.54 | 0.185 | 18.7 | P2 | 0.43 | 0.32 | 6.9 |
| | | | | | P5 | 0.27 | 0.20 | 4.3 |
| | | | | | P8* | 0.20 | 0.15 | 2.3 |
| A4-C2.8 | 0.015 | 2.77 | 0.180 | 18.2 | P2 | 0.41 | 0.31 | 5.1 |
| | | | | | P5 | 0.31 | 0.23 | 3.8 |
| | | | | | P8* | 0.20 | 0.15 | 2.5 |
| * deposition was | s observe | d at this loc | cation | | | | | |

2.2 Data acquisition

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At the start of each run, the water temperature was measured with a thermometer and the water 179 depth was measured with a ruler at P8. A vertical rack of siphon tubes was used to 180 synchronously collect 60 ml samples over a duration of 2 minutes at five different heights in the water column and at three locations for the decelerating (P3, P5, P9) and accelerating (P1, P5, 182 P7) flows (Fig. 2b, c). The three locations covered the longest lengths possible in the flume for 183 development of either decelerating or accelerating flow. Hence, the measurement locations 184 included the first measurement point upstream of the tapering section (P3 for decelerating flow, P7 for accelerating flow), the middle of the tapering section (P5) and the furthest measurement 186 point downstream of the tapering section (P9 for decelerating flow and P1 for accelerating flow). 187 The collected samples were weighed and dried to determine their volumetric clay concentration. The horizontal flow velocity was measured at nine locations along the flume using Ultrasonic 189 Velocity Profilers facing upstream (Fig. 2b, c) (Takeda, 1991, Best et al., 2001). Ultrasonic 190 Velocity Profilers measure flow velocity using the Doppler shift, which relies on the use of pulsed ultrasound echography. A short emission of ultrasound is transmitted from a profiler, and 192 the same profiler receives the echo reflected from suspended particles in the flow. To determine 193 the flow velocity, the Doppler shift frequency is determined from several repeated ultrasound 194 pulses. In these experiments, five 4 MHz probes were stacked on top of each other with a 195 196 distance of 14 mm between their centres. The probes collected velocity data for 500 cycles with a 50 ms delay between probes to avoid measurement interference. The probe array was shifted 197 vertically to three different heights during the experiment to cover the full flow depth, resulting 198 in a total of 15 measurement elevations per location (Fig. 2c). Depending on the experimental 199 conditions, these settings resulted in measurement durations of 174 to 330 s at a temporal 200 resolution of 2.9 to 1.5 Hz. Velocity measurements taken at 0.03 to 0.05 m from the probe head were used in the analysis. An overview of the settings of the Ultrasonic Velocity Profilers used 202 in these experiments is provided in the Supporting Information. 203

2.3 Data processing

Artificial noise was removed from the velocity signal by eliminating values three standard deviations away from a temporal moving mean measured over 31 datapoints. On average, these spikes accounted for less than 3% of the data. Datapoints were excluded where deposition occurred. The temporal mean flow velocity, \overline{U} , and its standard deviation, RMS(u'), were then calculated from the time series of instantaneous velocity data at each measurement height (Baas et al., 2009):

$$\overline{U} = \frac{1}{n} \sum_{i}^{n} u_{i} \tag{1}$$

$$RMS(u') = \sqrt{\frac{1}{n} \sum_{i}^{n} (u_i - \overline{U})^2}$$
 (2)

where n is the number of velocity measurements. The coefficient of variation is used as a 211

dimensionless measure for turbulence intensity (e.g. Baas et al. 2009): 212

$$RMS(u')_0 = \frac{RMS(u')}{\overline{U}} \cdot 100 \tag{3}$$

- Depth-averaged velocity was calculated by integrating the time-averaged velocities over the
- depth. The integral was numerically evaluated; velocities were set to zero at the bed and
- velocities at the water surface were assumed to have the same value as the first measurement
- 216 position below that level:

$$\overline{\overline{U}} = \frac{1}{h_0} \int_0^{h_0} \overline{U} dz \tag{4}$$

- 217 where z is height above the bed. Depth-averaged turbulence intensity was calculated by
- integrating the turbulence intensity values over the depth.

$$\overline{RMS(u')_0} = \frac{1}{h_0} \int_0^{h_0} RMS(u')_0 \, dz \tag{5}$$

- In the rare occasion that the reflected signal strength of a Ultrasonic Velocity Profiler is not
- sufficient to collect accurate velocity measurements, the velocity measurements can result in
- 222 unexpected strong velocity fluctuations. A moving mean is not guaranteed to remove these errors
- and a second stage of data cleaning is required. These outliers in the processed velocity dataset
- were excluded as follows. Data was identified as an outlier when either the flow velocity, \overline{U} , or
- its standard deviation RMS(u'), was 40% higher or lower than the median value of the six
- 226 immediately surrounding measurement points from the nearest upstream and downstream
- 227 locations:

$$\frac{\left| median\left(\overline{U}_{j-1,i-1}, \overline{U}_{j,i-1}, \overline{U}_{j+1,i-1}, \overline{U}_{j-1,i+1}, \overline{U}_{j,i+1}, \overline{U}_{j+1,i+1} \right) - \overline{U}_{j,i} \right|}{\overline{U}_{j,i}} \cdot 100 > 40$$
 (6)

- with i = point, j = height. Here, the median was used to avoid weighting from outliers. At the
- outer locations, P1 and P9, the points in the narrow (P2 and P3) or wide (P7 and P8) section were
- used to include a sufficient number of measurement points in the determination of the median,
- e.g. for outer location P1:

$$\frac{\left| median\left(\overline{U}_{j-1,P2},\overline{U}_{j,P2},\overline{U}_{j+1,P2},\overline{U}_{j-1,P3},\overline{U}_{j,P3},\overline{U}_{j+1,P3}\right)-\overline{U}_{j,P1}\right|}{\overline{U}_{j,P1}}\cdot 100 > 40$$

$$(7)$$

- Near the bed, larger changes in \overline{U} and RMS(u') are likely and therefore, the lowest measurement
- elevation was excluded from this outlier analysis. To make sure no outliers are left near the bed,
- 234 the lowest measurement elevation was compared only to the nearest upstream and downstream
- locations at the lowest measurement elevation. The second stage of data cleaning, discarded as
- 236 little as 1% and up to 7% of the datapoints from an experimental dataset. To maintain enough
- 237 datapoints over the depth, the full measurement location (P1-P9) was deemed invalid if >50% of
- the data was classified as outliers over the full flow depth. The bed height, z_h , was defined as the
- lowest valid measurement elevation. To compare the same elevation in different flows, the flows
- are plotted against normalized height adjusted to the deposit level.

$$\tilde{z} = (z - z_b)/h_0 \tag{8}$$

- Following Wan (1982), the dynamic viscosity, $\eta [N/(s/m^2)]$, of the suspensions was estimated
- 242 from the measured suspended sediment concentration:

$$\eta = 0.001 + 0.206 \left(\frac{C}{100}\right)^{1.68} \tag{9}$$

243 Then, the Reynolds number was calculated as:

$$Re = \frac{\overline{\overline{U}}h_0}{v_e} \tag{9}$$

- where, the effective viscosity of the suspension, v_e , was calculated from the ratio of dynamic
- viscosity over the density of the clay suspension, ρ_m :

$$v_e = \eta/\rho_m \tag{10}$$

- 246 The identified adaptation length, L, and time scales, T, are calculated in dimensionless form with
- 247 the standing water depth as characteristic length scale and the discharge as characteristic time
- scale, for which the velocity at P2 is representative.

$$L = l/h_0 [-] \tag{11}$$

$$T = t \cdot h_0 / U_{P2} [-] \tag{12}$$

- 249 where I is the identified adaptation length in the flume and t the identified adaptation time in the
- 250 flume.
- 251 3 Results
- The results section provides an overview of the collected measurements. This includes
- suspended sediment concentrations (Section 3.1) and streamwise velocity and turbulence
- intensity profiles along the flume for decelerating flows (Section 3.2) and accelerating flows
- 255 (Section 3.3).
 - 3.1 Clay concentration
- 257 The suspended sediment concentrations for the decelerating flows were nearly uniform over the
- 258 flow depth (Fig. 3a). The exception is run D3-C0.9, which contained a higher clay concentration
- at the lowest sampling point in the wide section (P9) of the flume. This may be explained as D3-
- 260 C0.9 has the slowest recorded velocity at P9, of the decelerating flows, and thus the greatest
- likelihood for deposition from suspension of the cohesive sediment (Fig. 5a). The suspended
- sediment concentrations for the accelerating flows were non-uniform over the flow depth, with
- 263 higher near-bed sediment concentrations, particularly in the wide section of the flume (Fig. 3b).
- These higher concentrations were in the deposit level of the flows ($\tilde{z} < 0$).

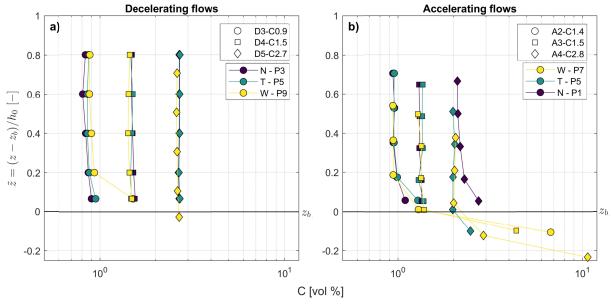


Figure 3. Vertical profiles of volumetric sediment concentration against normalized bed height adjusted to the deposit (eq 8) for the a) decelerating and b) accelerating clay-laden flows. The measurement locations are indicated in the order of the flow direction, where N, T and W denote narrow, tapering, and wide sections, respectively.

3.2 Decelerating flows

3.2.1 Clear water flows

Figure 4a shows the time-averaged streamwise velocity profiles (\overline{U}) and the depth-averaged velocity magnitudes ($\overline{\overline{U}}$) along the flume for the decelerating clear-water flow D1-C0.0. Upstream, in the narrow section of the flume (P1 to P3; Fig. 2b), the depth-averaged velocity shows that the flow is nearly uniform. The velocity decreases progressively as the width of the flume increases (P4 to P6) and continues to decrease more gradually in the wide section of the flume (P7 to P9). At the end of the flume (P9), uniform conditions are established in the lower half of the flow, but they are not fully established in the upper half. Figure 4b shows the velocities along the flume for the lower-discharge decelerating flow D2-C0.0 (Table 1). The depth-averaged velocities show a comparable pattern to flow D1-C0.0 (Fig. 4a, b).

Figures 4c and 4d show the time-averaged streamwise turbulence intensity profiles $(RMS(u')_0)$ and the depth-averaged turbulence intensities $(\overline{RMS(u')_0})$ along the flume for D1-C0.0 and D2-C0.0, respectively. The depth-averaged turbulence intensity values of both flows are nearly uniform in the narrow section (P1 to P3). The turbulence intensities decrease away from the bed in the narrow section (Fig 4c, d). As the velocity decreases in the widening section (P4 to P6), turbulence intensity increases near the bed, while also progressively increasing upwards in the flow downstream. In both flows, this results in an increase in vertical gradient of turbulence intensity in the widening section followed by a decrease in the vertical gradient in the wide section. The depth-averaged turbulence intensity at P9 is 4.0 times higher than at P2 for D1-C0.0 (Fig. 4c) and 3.7 times higher for D2-C0.0 (Fig. 4d), despite the decrease in velocity. Similar increases in turbulence intensity have been observed before in clear water decelerating flows

(Kironota and Graf, 1995; Qingyang, 2009). Towards the end of the wide section, at P9, the turbulence intensities remain non-uniform, suggesting that the length of the flume is insufficient to establish equilibrium after the widening section.

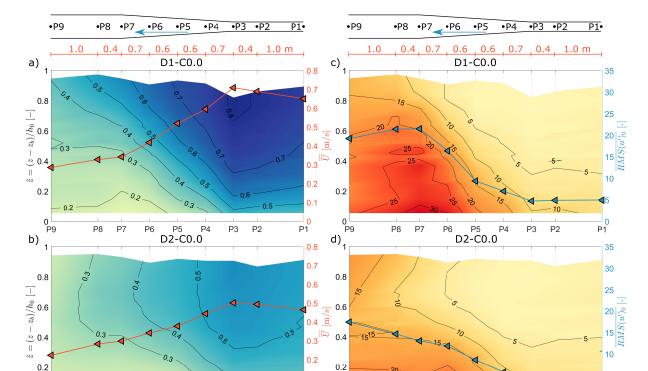


Figure 4. Depth-averaged velocity magnitudes ($\overline{\overline{U}}$) and time-averaged streamwise velocity profiles ($\overline{\overline{U}}$) along the flume for the decelerating clear water flows a) D1-C0.0 and b) D2-C0.0. Depth-averaged turbulence intensities ($\overline{RMS(u')_0}$) and time-averaged streamwise turbulence intensity profiles ($RMS(u')_0$) along the flume for flows c) D1-C0.0 and d) D2-C0.0.

0.1 0 P1

Р3

0.5

0 └ P9

0

P8

10

P6

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 $RMS(u')_0$ [-]

РЗ

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3.2.2 Clay-laden flows

0 └ P9

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Figures 5a, 5b and 5c show the time-averaged streamwise velocity profiles (\overline{U}) and the depth-averaged velocity magnitudes ($\overline{\overline{U}}$) along the flume for the clay-laden decelerating flows D3-C0.9, D4-C1.5 and D5-C2.7, respectively. Figures 5d, 5e and 5f show the time-averaged streamwise turbulence intensity profiles ($RMS(u')_0$) and the depth-averaged turbulence intensities ($\overline{RMS(u')_0}$) along the flume for the same flows. In the narrow section (P1 to P3), the depth-averaged velocities are nearly uniform for each decelerating clay-laden flow. The depth-averaged velocities for each flow decrease along the widening section similarly, albeit with a slightly higher rate of decrease for flow D4-C1.5. In the wide section (P7 to P9), the depth-averaged velocities are lowest and nearly uniform.

The depth-averaged turbulence intensity values are nearly uniform in the narrow section (P1 to P3) (Figs 5d, 5e and 5f); the turbulence intensities decrease away from the bed. As the velocity

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| 316 | decreases in the widening section (P4 to P6), the turbulence intensity increases, initially near the |
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| 317 | bed, and then progressively higher in the flow downstream. This results in an increase in vertical |
| 318 | gradient of turbulence intensity in the widening section followed by a decrease in vertical |
| 319 | gradient into the wide section. Towards the end of the wide section, at P9, the turbulence |
| 320 | intensity shows a steep vertical gradient for flows D3-C0.9 and D4-C1.5. The turbulence |
| 321 | intensity for flow D5-C2.7 remains high between P7 and P9. Despite the decrease in velocity, the |
| 322 | depth-averaged turbulence intensity at P9 is 3.6 times higher than at P2 for D3-C0.9, 4.3 times |
| 323 | higher for D4-C1.5 and 1.8 times higher for D5-C2.7. Towards the end of the wide section, at |
| 324 | P9, the turbulence intensities remain non-uniform, suggesting that the length of the flume is |
| 325 | insufficient to establish equilibrium after the widening section. Despite, both the clear water and |
| 326 | clay-laden decelerating flows not reaching equilibrium flow conditions in the wide section, |
| 327 | distinct differences in patterns of increase in turbulence intensity can be identified, which |
| 328 | determines clay flow type, discussed below in Section 4.1. |
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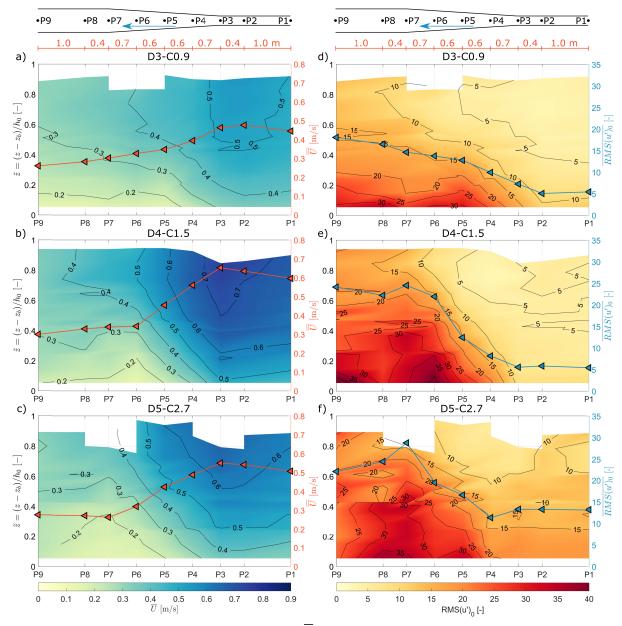


Figure 5. Depth-averaged velocity magnitudes ($\overline{\overline{U}}$) and time-averaged streamwise velocity profiles (\overline{U}) along the flume for the decelerating clay-laden flows a) D3-C0.9, b) D4-C1.5 and c) D5-C2.7. Depth-averaged turbulence intensities ($\overline{RMS(u')_0}$) and time-averaged streamwise turbulence intensity profiles ($RMS(u')_0$) along the flume for flows d) D3-C0.9, e) D4-C1.5 and f) D5-C2.7.

3.3 Accelerating flows

The flow direction was reversed to achieve accelerating conditions, so the flow direction was from left to right, i.e. from P9 to P1 (cf. Fig. 2a and 2b).

3.3.1 Clear water flows

Figure 6a shows the time-averaged streamwise velocity profile (\overline{U}) and the depth-averaged velocity magnitude (\overline{U}) along the flume for the accelerating clear-water flow A1-C0.0. Upstream, in the wide section of the flume (P9 to P7; Fig. 2b), the depth-averaged velocity shows that the flow is nearly uniform. The flow accelerates progressively as the width of the flume decreases (P6 to P4) and nearly uniform flow re-establishes in the narrow section (P3 to P1).

Figure 6b shows the time-averaged streamwise turbulence intensity profile $(RMS(u')_0)$ and the depth-averaged turbulence intensities $(RMS(u')_0)$ along the flume for flow A1-C0.0. The depth-averaged turbulence intensity values are nearly uniform in the wide section (P9 to P7). The turbulence intensity values decrease as the velocity increases in the narrowing section (P6 to P4) and remain nearly uniform in the narrow section (P3 to P1). The depth-averaged turbulence intensity at P1 is lower by a factor of 0.3 than at P8. The velocity increases towards the narrow section, but its standard deviation (RMS(u')) does not rise accordingly, which results in a

decrease in turbulence intensity $(RMS(u')_0)$. Similar decreases in turbulence intensity have been observed before in accelerating clear water flows (Cardoso et al., 1991).

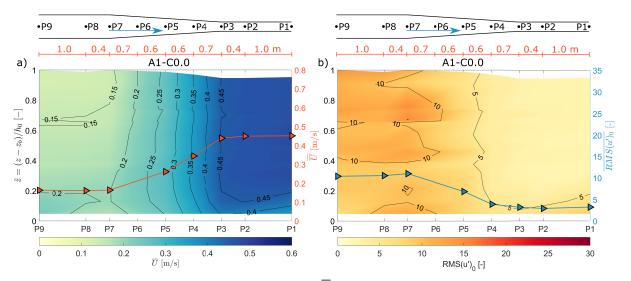
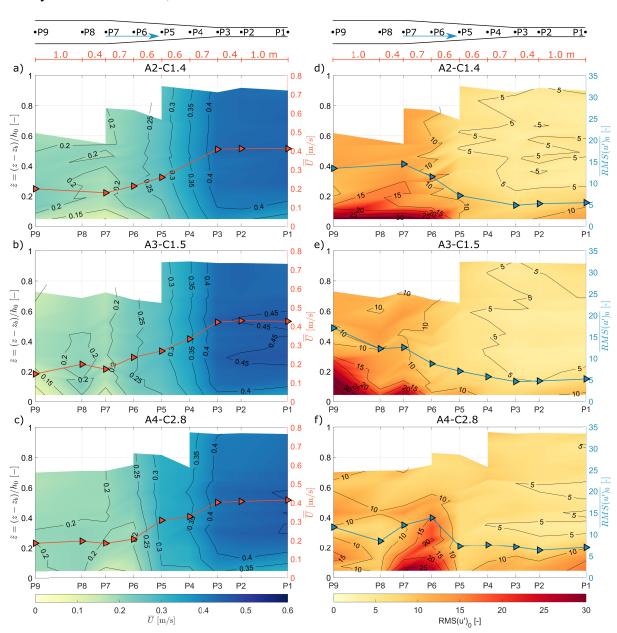


Figure 6. a) Depth-averaged velocity magnitudes (\overline{U}) and time-averaged streamwise velocity profiles (\overline{U}) along the flume for the accelerating clear water flow A1-C0.0. b) Depth-averaged turbulence intensities ($\overline{RMS(u')_0}$) and time-averaged streamwise turbulence intensity profiles ($RMS(u')_0$) along the flume for flow A1-C0.0.

3.3.2 Clay-laden flows

Figures 7a, 7b and 7c show the time-averaged streamwise velocity profiles (\overline{U}) and the depth-averaged velocity magnitudes ($\overline{\overline{U}}$) along the flume for the clay-laden accelerating flows A2-C1.4, A3-C1.5 and A4-C2.8, respectively. Figures 7d, 7e and 7f show the time-averaged streamwise turbulence intensity profiles ($RMS(u')_0$) and the depth-averaged turbulence intensities ($\overline{RMS(u')_0}$) along the flume for the same flows. Upstream in the wide section (P9 to P7; fig 2b), the depth-averaged velocity shows that the flow is nearly uniform. The flow accelerates progressively as the width of the flume decreases (P6 to P4) and nearly uniform flow reestablishes in the narrow section (P3 to P1).

In the wide section (P9 to P7), where the velocity is low, the depth-averaged turbulence intensities of all three clay flows are higher than in the narrowing and narrow sections, where the velocities are higher (Fig. 7d, 7e and 7f). Towards the base of the flow, the turbulence intensity shows a steep vertical gradient in the wide section, with especially high turbulence intensity towards the base of flows A2-C1.4 and A3-C1.5. Notably, the turbulence intensity in the bottom half of the flow at P9 and P8 in the wide section of the flume is lower for flow A4-C2.8 (Fig. 7f) than for flows A2-C1.4 (Fig 7d) and A3-C1.5 (Fig 7e). The turbulence intensity values are high around P7 for flow A4-C2.8. The depth-averaged turbulence intensity values for all three flows decrease as the velocity increases in the narrowing section (P6 to P4) and remain nearly uniform in the narrow section (P3 to P1). The depth-averaged turbulence intensity at P1 is 0.4 times the intensity at P8 for A2-C1.4, 0.4 times for A3-C1.5 and 0.8 times for A4-C2.8.



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- Figure 7. Depth-averaged velocity magnitudes ($\overline{\overline{U}}$) and time-averaged streamwise velocity
- profiles (\overline{U}) along the flume for the accelerating clay-laden flows a) A2-C1.4, b) A3-C1.5 and c)
- A4-C2.8. Depth-averaged turbulence intensities $(\overline{RMS(u')_0})$ and time-averaged streamwise
- turbulence intensity profiles $(RMS(u')_0)$ along the flume for flows d) A2-C1.4, e) A3-C1.5 and
- 388 f) A4-C2.8.

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4 Discussion

- The discussion includes the interpretation of downstream changes in clay flow types in the
- experimental runs (Section 4.1). Based on the distance between the different clay flow types in
- the flume, the length scale of adaptation of clay flows is assessed in Section 4.2. The length
- scales of decelerating and accelerating clay-laden flows are compared and further implications of
- the present study are discussed in Section 4.3.

4.1 Clay flow types

profile over most of the flow depth.

To determine the clay flow types at the nine measurement locations along the flume initially without influences of flow deceleration or acceleration, the difference in turbulence intensity is assessed between clay-laden flows and clear water flows. Figure 8 shows the profiles of turbulence intensity $(RMS(u')_0)$ for the five clay flow types identified by Baas et al. (2009) with an added dashed line indicating the turbulence intensity profile of a clear water turbulent flow. Additionally, Figure 8 shows the difference profiles of turbulence intensity ($\Delta RMS(u')_0$) between the five clay flow types and clear water turbulent flow. When compared with turbulent clear water flow, the difference in turbulence intensity is negligible if the clay-laden flow is classified as turbulent flow. Turbulence-enhanced transitional flows show higher turbulence intensity over the full flow depth and thus, if compared with turbulent flow, the difference profile $(\Delta RMS(u')_0)$ results in positive values over the full flow depth. The plug flow formation below the surface for lower transitional plug flows results in negative $\Delta RMS(u')_0$ values below the surface in the difference profile. However, increased $\Delta RMS(u')_0$ values are found near the bed, since lower transitional plug flow exhibits increased near-bed turbulence. With the thickening of the plug flow in upper transitional plug flows, negative $\Delta RMS(u')_0$ values expand towards the bed. Fully suppressed turbulence in quasi-laminar plug flows results in a negative difference

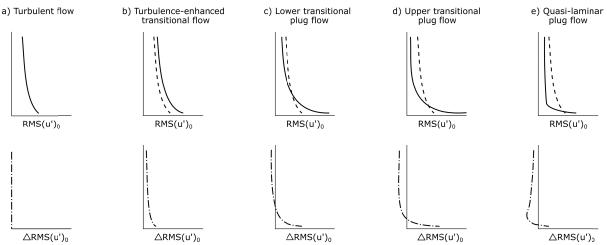


Figure 8. Upper row: schematic model of turbulence intensity profiles (RMS(u')₀), divided into five different clay flow types after the classification scheme of Baas et al. (2009), where the dashed line in b-e indicates the turbulence intensity profile for turbulent flow. Lower row: schematic model of the difference in time-averaged streamwise turbulence intensity profiles $(\Delta RMS(u')_0)$ between clay flow types and clear water turbulent flows.

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446 447 Figures 9a and 9b show the difference in time-averaged streamwise turbulence intensity profiles $(\Delta RMS(u')_0)$ and in depth-averaged turbulence intensities $(\Delta RMS(u')_0)$ along the flume for decelerating flows D3-C0.9 and D5-C2.7 versus flow D2-C0.0 and Table 2 shows an overview of the identified clay-flow types. Differences between the normalized turbulence intensity, $RMS(u')_0$, over the normalized flow depth, \tilde{z} , allows the assessment of relative influence of clay concentration on non-uniform decelerating flow conditions and allows the interpretation of clay flow types. Since the relative influence of clay concentration is assessed on the flow dynamics, the same flow types can be identified by comparison of the decelerating clay-laden flows between either clear water flows (D1-C0.0 and D2-C0.0). Here, flow D2-C0.0 is selected for the comparison, because the depth-averaged velocity in the narrow section (P2) before decelerating the flow is more comparable to flow D3-C0.9 and D5-C2.7 (Table 1). Upstream, in the narrow section (P1 to P3; Fig. 2b), the turbulence intensity values of flow D3-C0.9 are comparable with the clear-water flow D2-C0.0, i.e., the $\Delta RMS(u')_0$ values are relatively close to zero. This suggests turbulent flow, unaffected by the presence of the suspended clay (Fig. 8; Table 2). As the flow decelerates in the widening section (P4 to P6), the $\Delta RMS(u')_0$ values increase to 10 in the lower half of the flow and to 2.5 in the upper half of the flow. This is typical of turbulenceenhanced transitional flow (Fig. 8; Baas et al., 2009); under these conditions the presence of the clay is inferred to cause a thickening of the viscous sublayer and the development of an internal shear layer with associated enhancement of turbulence (Best and Leeder, 1993; Li and Gust, 2000; Baas and Best, 2002). In the wide section (P7 to P9), the $\Delta RMS(u')_0$ values remain above zero in the bottom half of flow D3-C0.9 and they are zero or below zero in the top half of the flow. These negative $\Delta RMS(u')_0$ values suggest the onset of plug development in flow D3-C0.9, i.e., lower transitional plug flow (Fig. 8; Baas et al., 2009). Flows D3-C0.9 and D4-C1.4 show comparable $\Delta RMS(u')_0$ patterns (Fig. 5d and 5e), such that the same flow types can be identified.

In the narrow section (P1 to P3), the increased clay concentration in flow D5-C2.7 is inferred to 448 cause the observed positive $\Delta RMS(u')_0$ values (Fig. 9b). This suggests that flow D5-C2.7 begins 449 as a turbulence-enhanced transitional flow (Fig. 8; Table 2; Baas and Best, 2002). The 450 $\Delta RMS(u')_0$ values progressively increase through the widening section and beyond, suggesting 451 452 the development of stronger turbulence-enhanced transitional flow (Baas et al., 2009). While the mean velocity profile of flow D5-C2.7 appears reliable, the heterogeneous vertical pattern of 453 $\Delta RMS(u')_0$ above a relative depth of 0.4 at position P9 (Fig. 9b) may arise from artefacts in the 454 RMS(u') measurements of this flow. This hinders a reliable inference of flow type at this 455 456 location, but the decrease in $\Delta RMS(u')_0$ below the relative depth of 0.4 between P8 and P9 combined with a decrease in $\Delta RMS(u')_0$ near the top of the flow between P8 and P7 may 457 indicate a change from turbulence-enhanced transitional flow via lower transitional plug flow to 458 459 upper-transitional plug flow in the wide section (P7 to P9).

Table 2. Identified clay flow types at the measurement positions in the flume, P9 to P1. The labelling of the clay flow types in the table is as follows: TF = Turbulent flow; TETF = Turbulence-enhanced transitional flow; LTPF = Lower transitional plug flow; UTPF = Upper transitional plug flow; QLPF = Quasi-laminar plug flow.

| Experimental | Clay flow type | | | | | | | | |
|-------------------|----------------|------|------|------|------|------|------|------|------|
| run | | | | | | | | | |
| | P9 | P8 | P7 | P6 | P5 | P4 | P3 | P2 | P1 |
| Decelerating flow | | | | | | | | | |
| D3-C0.9 | LTPF | LTPF | LTPF | TETF | TETF | TETF | TF | TF | TF |
| D5-C2.7 | UTPF | LTPF | TETF |
| Accelerating flow | | | | | | | | | |
| A2-C1.4 | LTPF | LFTP | LTPF | LTPF | TETF | TETF | TETF | TETF | TETF |
| A4-C2.8 | UTPF | UTPF | LTPF | LTPF | LTPF | TETF | TETF | TETF | TETF |

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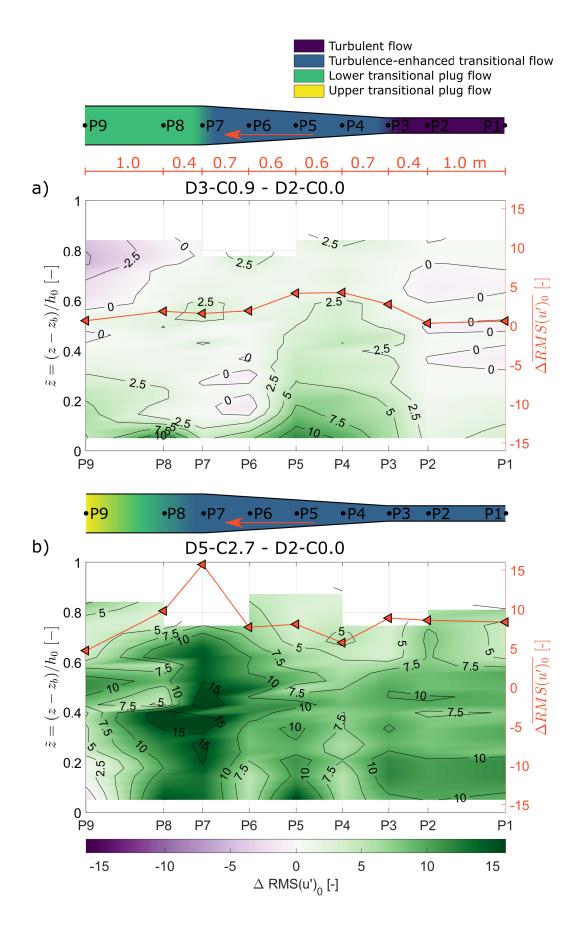


Figure 9. Difference in depth-averaged turbulence intensities $(\Delta \overline{RMS(u')_0})$ and time-averaged streamwise turbulence intensity profiles $(\Delta RMS(u')_0)$ along the flume for decelerating flows a) D3-C0.9 minus D2-C0.0 and b) D5-C2.7 minus D2-C0.0.

 Figures 10a and 10b show the difference in time-averaged streamwise turbulence intensity profiles $(\Delta RMS(u')_0)$ and in depth-averaged turbulence intensities $(\Delta RMS(u')_0)$ along the flume for accelerating flows A2-C1.4 and A4-C2.8 versus flow A1-C0.0, and Table 2 shows an overview of the identified clay-flow types. Differences between the normalized turbulence intensity, $RMS(u')_0$ over the normalized flow depth z allows the assessment of relative influence of clay concentration on non-uniform accelerating flow conditions and allows the interpretation of clay flow types. Upstream, in the wide section and at the start of the narrowing section (P9 to P6), $\Delta RMS(u')_0$ values are relatively close to zero in the upper half of the flow and increase downwards to 15 in the lower half of flow A2-C1.4. The high near-bed $\Delta RMS(u')_0$ values, in combination with the low values in the upper half of the flow, are typical of lower transitional plug flow (Fig. 8; Baas et al., 2009). As the flow accelerates through the narrowing section (P6 to P4), the near bed $\Delta RMS(u')_0$ values progressively decrease from 10 to c. 2.5. In the narrow section (P3 to P1), the absolute turbulence intensity values of flow A2-C1.4 are low (Fig. 7d), but the $\Delta RMS(u')_0$ values are increased to around 2.5. This enhanced turbulence intensity suggests weakly turbulence-enhanced or turbulent flow (Fig. 8). Flow A3-C1.5 shows comparable turbulence intensity patterns and values (Fig. 7d and 7e) and similar flow types can be identified.

Upstream, in the wide section (P9 to P8), $\Delta RMS(u')_0$ values are up to 2.5 in the lower half of the flow and down to -2.5 in the upper half for flow A4-C2.8 (Fig. 10b). This profile suggests upper transitional plug flow, where turbulence enhancement near the bed is lower than for lower transitional plug flows (Fig.8; cf., flow A2-C1.4 in Fig. 10a). Similar to flow A2-C1.4, $\Delta RMS(u')_0$ values of flow A4-C2.8 between P7 and P6 are relatively close to or below zero in the upper half of the flow and are as high as 15 in the lower half of the flow, suggesting lower transitional plug flow (Fig. 10b). Between P4 and P1, the depth-averaged $\Delta RMS(u')_0$ values are between 2.5 and 5 and vertical $\Delta RMS(u')_0$ profiles are strictly positive, suggesting turbulence-enhanced transitional flow (Fig. 8).

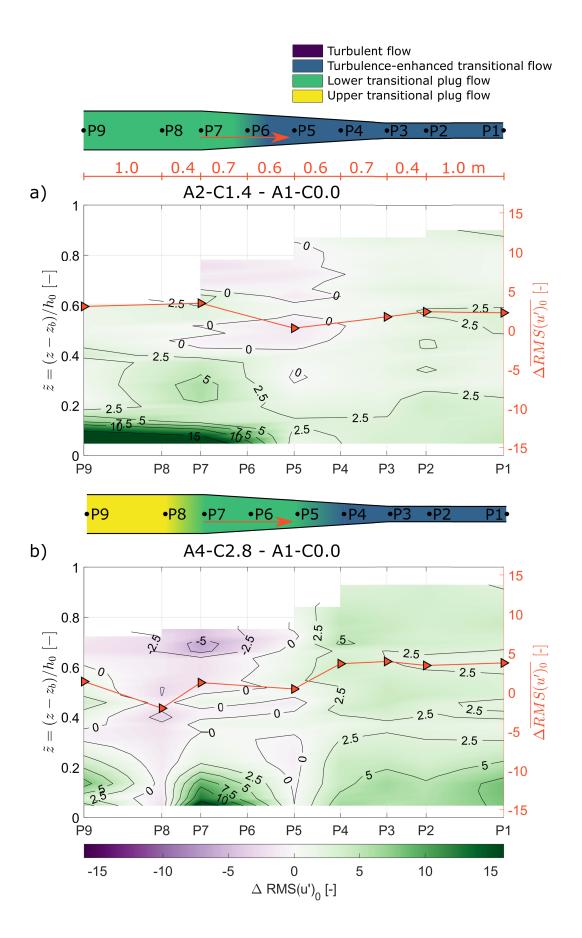


Figure 10. Difference in depth-averaged turbulence intensities $(\Delta \overline{RMS(u')_0})$ and time-averaged streamwise turbulence intensity profiles $(\Delta RMS(u')_0)$ along the flume for decelerating flows a) A2-C1.4 minus A1-C0.0 and b) A4-C2.8 minus A1-C0.0.

4.2 Observed adaptation length scales

The length scales needed by clay flows to adapt to non-uniform conditions can be estimated using the data presented in Fig. 9 and 10. The length scales are based on the identified clay-flow types (Table 2) and the distance between the measurement points at locations where a change in velocity is experienced, i.e. these estimations involve length scales downstream of the start of the widening section for the decelerating flows and the narrowing section of the accelerating flows, as well as in the wide section for the decelerating flows and in the narrow section for the accelerating flows. The adaptation length scale in the wide (decelerating flow) or narrow section (accelerating flow) is determined by the distance required to develop (nearly) uniform conditions. The adaptation length and time scales are made dimensionless using the standing water depth and the depth-averaged velocity at P2 as characteristic length and time scales (eq 11,12).

For decelerating flows, the adaptation length scales are determined at the widening section and in the wide section as the flow adapts to the change in velocity. As the flow decelerated at the start of the widening section (P3), flow D3-C0.9 changed from turbulent flow to turbulence-enhanced transitional flow, without a significant adaptation length at this position (Fig. 9a; Table 3). Throughout the wide section (P7 to P9), the flow adjusted from turbulence-enhanced transitional flow to lower transitional plug flow. Towards the end of the wide section, at P9, $\Delta RMS(u')_0$ remained non-uniform, suggesting that the length of the flume was insufficient to establish uniform conditions after the widening section (Fig. 9a). Hence, the minimum adaptation length needed to change from turbulence-enhanced flow to lower transitional plug flow was 1.4 m, the full distance between P7 and P9 (Fig. 2b). At the depth-averaged velocity of

0.28 m/s in the wide section (Table 1), this adaptation length corresponds to a minimum adaptation time of 5.0 s.

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Flow D5-C2.7 started to change from a relatively weak to a stronger turbulence-enhanced transitional flow at position P4, i.e., 0.7 m into the widening section (Fig. 5f), whereas $\Delta \overline{RMS(u')_0}$ started to increase at P3 in flow D2-C0.0, i.e., at the start of the widening section (Fig. 4d). The maximum adaptation length this high-concentration clay flow needed after starting to experiencing flow widening was therefore 0.7 m (distance between P3 and P4, Fig. 2b). This is equivalent to an adaptation time of 1.4 s at a mean depth-averaged flow velocity of 0.52 m/s between P3 and P4 (Table 1). Flow D5-C2.7 changed from turbulence-enhanced transitional flow via lower transitional plug flow to upper transitional plug flow in the wide section (P7 to P9), without apparently reaching uniform flow conditions (Fig. 9b). This is equivalent to a minimum adaptation time of 5.2 s at a depth averaged flow velocity of 0.27 m/s (Table 1) through the 1.4-m long wide section (Fig. 2b).

For accelerating flows, the adaptation length scales are determined at start of the narrowing section and in the narrow section as the flow adapts to the change in velocity. Flow A2-C1.4 changed from lower-transitional plug flow at P6 to turbulence-enhanced transitional flow at P5 in the narrowing section. The distance between P6 and P5 is 0.6 m and with a depth-averaged

velocity of 0.26 m/s, this results in an adaptation time of 2.3 s. At the start of the narrow section, P3, flow A2-C1.4 established uniform turbulence-enhanced transitional flow (Fig 10a) and show no adaptation in the narrow section itself. Hence, within the spatial resolution of the experiments, the adaptation length in the narrow section was at or close to zero.

Flow A4-C2.8 started to change from upper transitional plug flow to lower transitional plug flow at the start of the narrowing section, at P7 and showed no signs of additional adaptation in the narrowing section (Fig. 10b) Hence, the change in clay flow type also lacked a significant delay at this location. At the start of the narrow section, P3, flow A4-C2.8 changed from lower transitional plug flow to turbulence-enhanced transitional flow. Flow A4-C2.8 established uniform turbulence-enhanced transitional flow at the start without additional adaptation in the narrow section. Hence, the change in clay flow type also lacked a significant delay at this location.

Table 3. *Observed dimensional and calculated dimensionless adaptation length scales, l and L, and time scales, t and T.*

| Experimental run | Location | Point(s) included in adaptation | Flow regimes | 1 | L | t | Т |
|------------------|-------------------|---------------------------------|--|------|------|------|-----|
| | | | | [m] | [-] | [s] | [-] |
| | | D | ecelerating flow | | | | |
| D3-C0.9 | Widening section | P3 | Turbulent flow to turbulence-enhanced transitional flow | 0 | 0 | 0 | 0 |
| | Wide section | P7 to P9 | Turbulence-enhanced transitional flow to lower transitional plug flow | ≥1.4 | 9.3 | ≥5.0 | 2.1 |
| D5-C2.7 | Widening section | P3 to P4 | Weak to strong turbulence- enhanced transitional flow | 0.7 | 4.7 | 1.4 | 0.4 |
| | Wide section | P7 to P9 | Turbulence-enhanced transitional flow to upper transitional plug flow | ≥1.4 | 9.3 | ≥5.2 | 1.4 |
| 1001 | 1 | | ccelerating flow | 106 | 10.5 | 100 | |
| A2-C1.4 | Narrowing section | P6 to P5 | Lower transitional plug flow to turbulence-enhanced transitional flow | 0.6 | 3.5 | 2.3 | 0.9 |
| | Narrow section | P3 | Uniform turbulence- enhanced transitional flow | 0 | 0 | 0 | 0 |
| A4-C2.8 | Narrowing section | P7 | Upper transitional plug flow to lower transitional plug flow | 0 | 0 | 0 | 0 |
| | Narrow section | Р3 | Lower transitional plug flow to turbulence- enhanced transitional flow | 0 | 0 | 0 | 0 |

4.3 Implications of adaptation length scales

Figure 11 shows an overview of the clay flow types in the experimental runs and the dimensionless adaptation length scales. The adaptation length and time scales show that the decelerating flows generally needed longer to adapt to the imposed non-uniform conditions than the accelerating flows (Fig. 11, Table 3). The largest adaptation lengths and times were at the end of the widening section in the decelerating flows, where the flows changed from turbulence-enhanced transitional flow to more cohesive lower and upper transitional plug flows. In contrast, the accelerating flows changed from the more cohesive lower transitional plug flow to turbulence-enhanced flow already in the narrowing section. These differences in adaptation length between the decelerating and accelerating flows can be explained by the fact that establishing cohesive bonds between clay particles, as in the decelerating flows, requires more time than breaking up these bonds, as in the accelerating flows.

Stronger turbulence attenuated flow types are identified in the clay flows with higher clay concentrations. It appears to take longer to establish a pervasive network of clay bonds, as in the change from turbulence-enhanced transitional flows to lower and upper transitional plug flow at the end of the widening section in the decelerating flows, than to establish a turbulence-enhanced transitional flow from a turbulent flow by reducing the flow velocity in low-concentration clay flows (e.g., Fig. 9a).

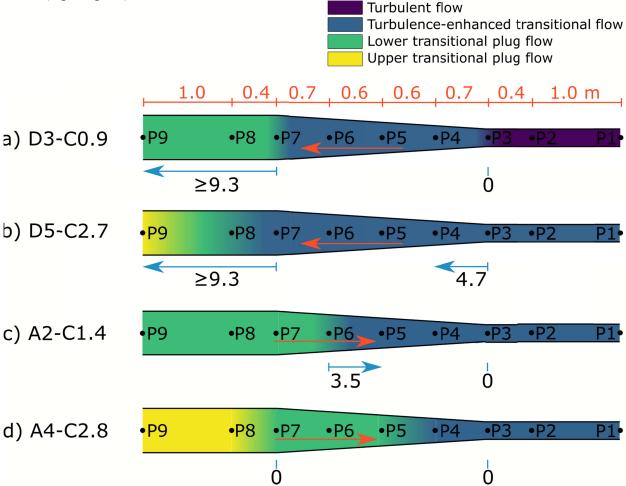


Figure 11. Identified clay flow types and observed dimensionless adaptation length scale, L.

The research focus here is on adaptation of flow dynamics of non-uniform clay-laden flows, but the length and time scales of flow adaptation can also be reflected in the depositional product (Dorrell and Hogg, 2012). Here, non-uniformity on spatial deceleration and acceleration in clay-laden open-channel demonstrates that these adaptation scales in mud-rich flows fundamentally differ between decelerating and accelerating regimes, due to the time required to form or break cohesive bonds between particles. These results are based on streamwise velocity measurements, due to the limitations of Ultrasonic Velocity Profilers, which are designed to work along a single beam. Further developments in technology are needed to fully resolve the turbulent motion of highly concentrated flows.

Additional research in the sedimentological record is required to determine how deposits of nonuniform clay suspension flows can be recognized in fluvial, estuarine and submarine systems. For example, after a sediment supply increase in a river following wild-fire related erosion (Renau et al., 2007; Sankey et al., 2017; Nyman et al., 2019), flow deceleration can occur following for example, a reduction in bed slope or widening of the river channel. The flow deceleration reduces the turbulent forces in the flow and allows the establishment of cohesive bonds between clay particles. The adaptation to stronger turbulence attenuated clay flow types requires time due to the formation of clay bonds and consequently, the deposits associated with the clay flow type form over the adaptation length scale downstream of the location of flow deceleration. In an industrial setting such as downstream of dam flushing or venting events flow acceleration can occur (Antoine et al., 2020), increasing the turbulent forces in the flow, which has the potential to break up bonds between clay particles. This study shows that the adaptation of the clay flow type to a stronger turbulent flow occurs more rapidly and consequently the associated deposits with clay flow type occur near the location of acceleration. Additionally, the different adaptation length and time scales are of particular relevance in interpreting the shape of submarine deposits, such as unconfined submarine lobes (Spychala et al., 2017) and hybrid event beds deposited around diaripirs (Davis et al., 2009; Patacci et al., 2014). It is anticipated that the depositional record of decelerating flows reflects the time scales required to form interparticle bonds, delaying the depositional response to the associated changes in flow conditions. For accelerating flows it is anticipated that changes in deposit properties associated with bond breakage occur more rapidly, such that they are more closely associated with the areas where acceleration occurs.

5 Conclusions

This research investigated the influence of suspended cohesive clay on changing flow dynamics under non-uniform flow conditions, using decelerating and accelerating open-channel flows in a recirculating flume. These flows may evolve through different clay flow types with different associated degrees of turbulence enhancement and attenuation depending on the clay concentration and whether the flows decelerate or accelerate. Decelerating flows have a longer adaptation time than accelerating flows, as establishing cohesive bonds between clay particles requires more time than breaking the clay bonds. This hysteresis is more pronounced for higher-concentration flows that change from the turbulence-enhanced transitional flow type to the lower and upper transitional plug flow types than for lower-concentration decelerating flows that change from the turbulent flow type to the turbulence-enhanced transitional flow type. Differences in adaptation time likely influence the distribution and character of deposit in

- sedimentary environments. The associated deposits with clay flow type of decelerating flows are
- 629 likely spread over a larger distance than of accelerating flow due to the elongated adaptation time
- of decelerating flows.

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Data Availability Statement

- The data collected during the physical experiments in preparation for this research is available at
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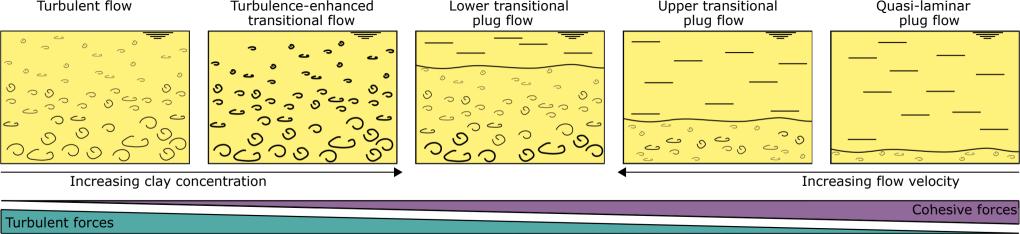
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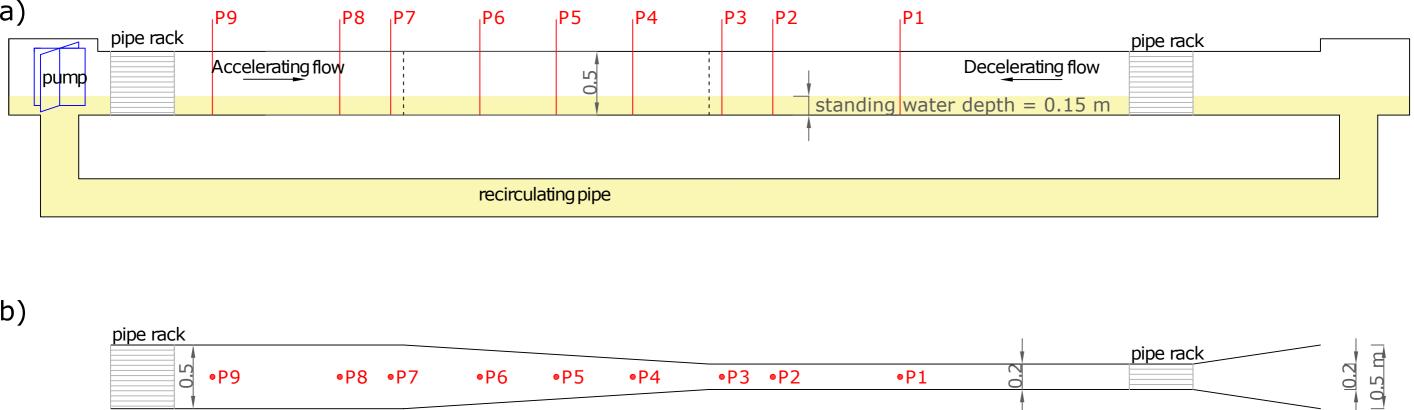
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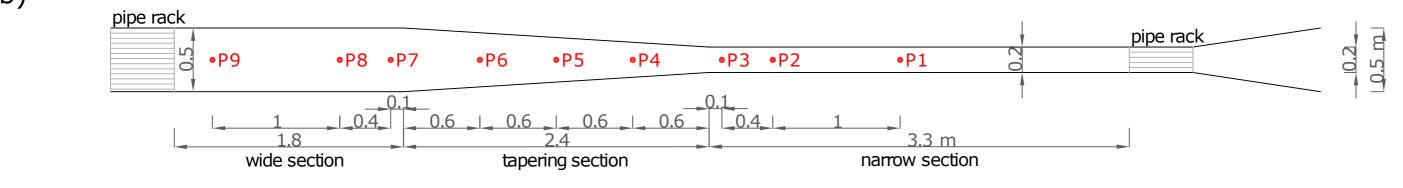
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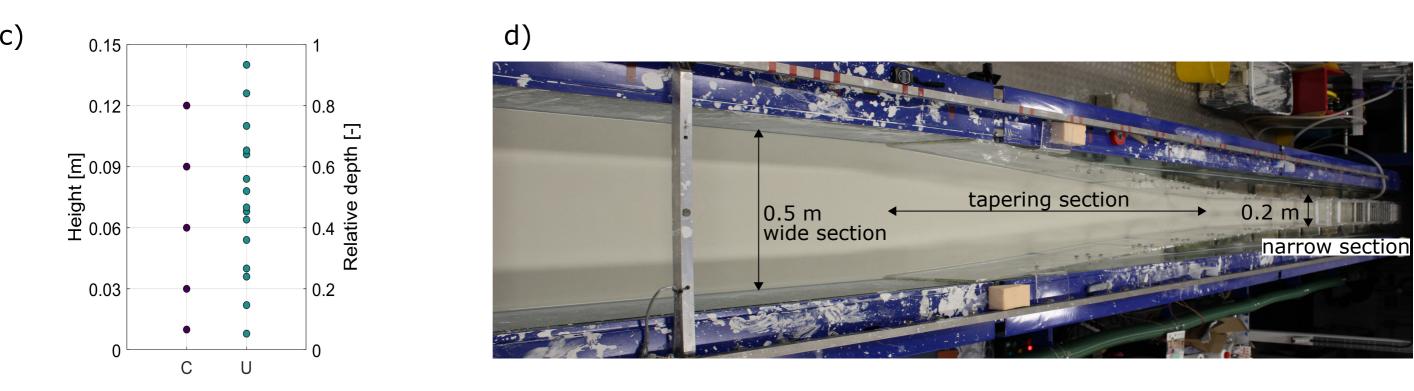
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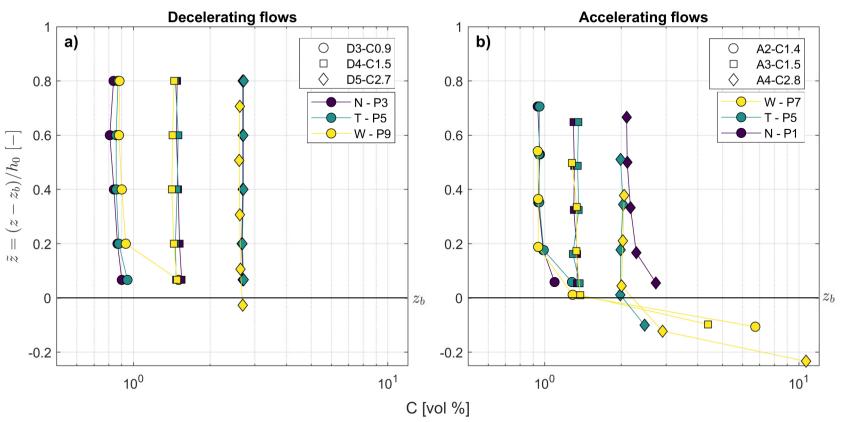
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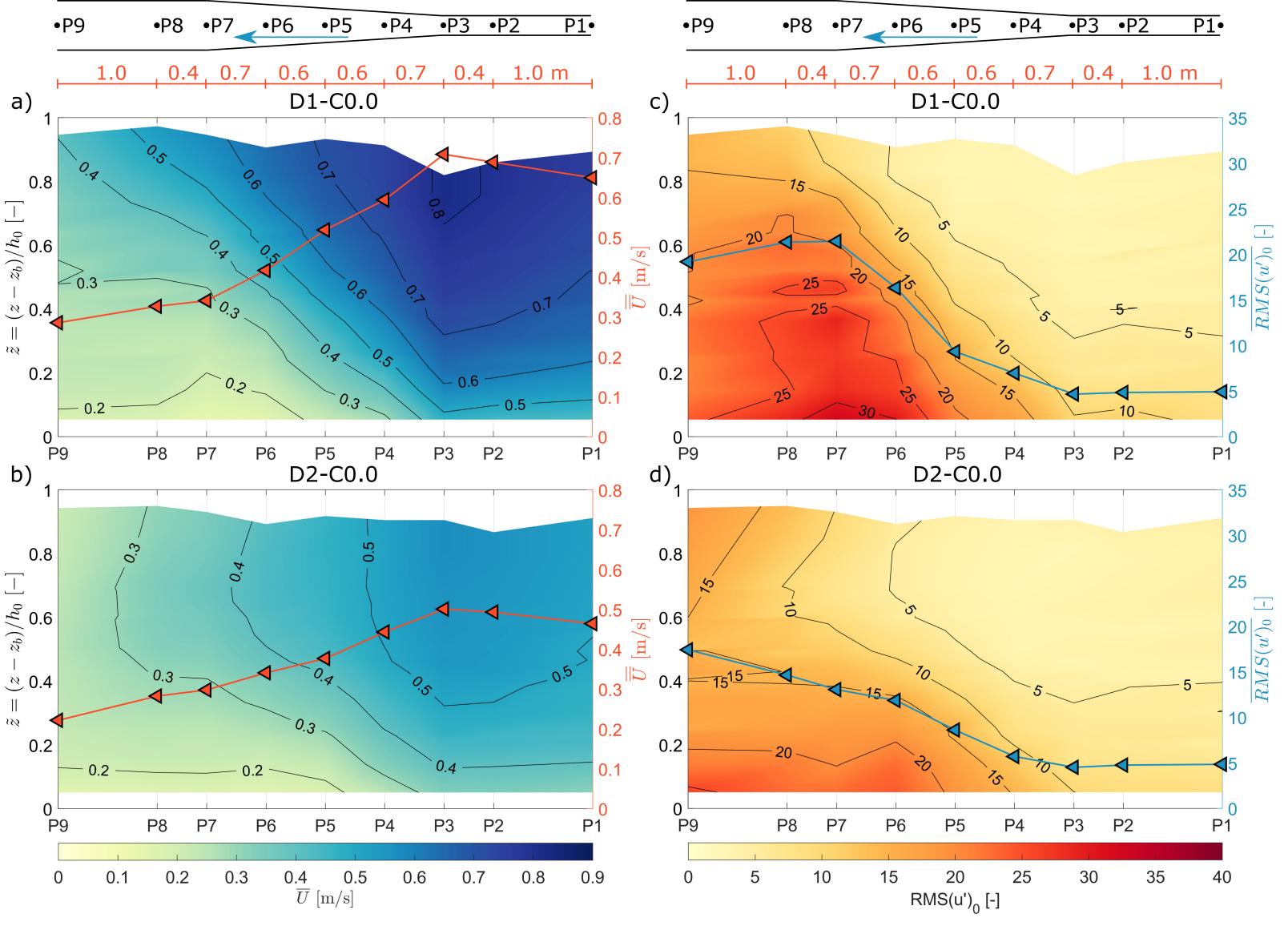




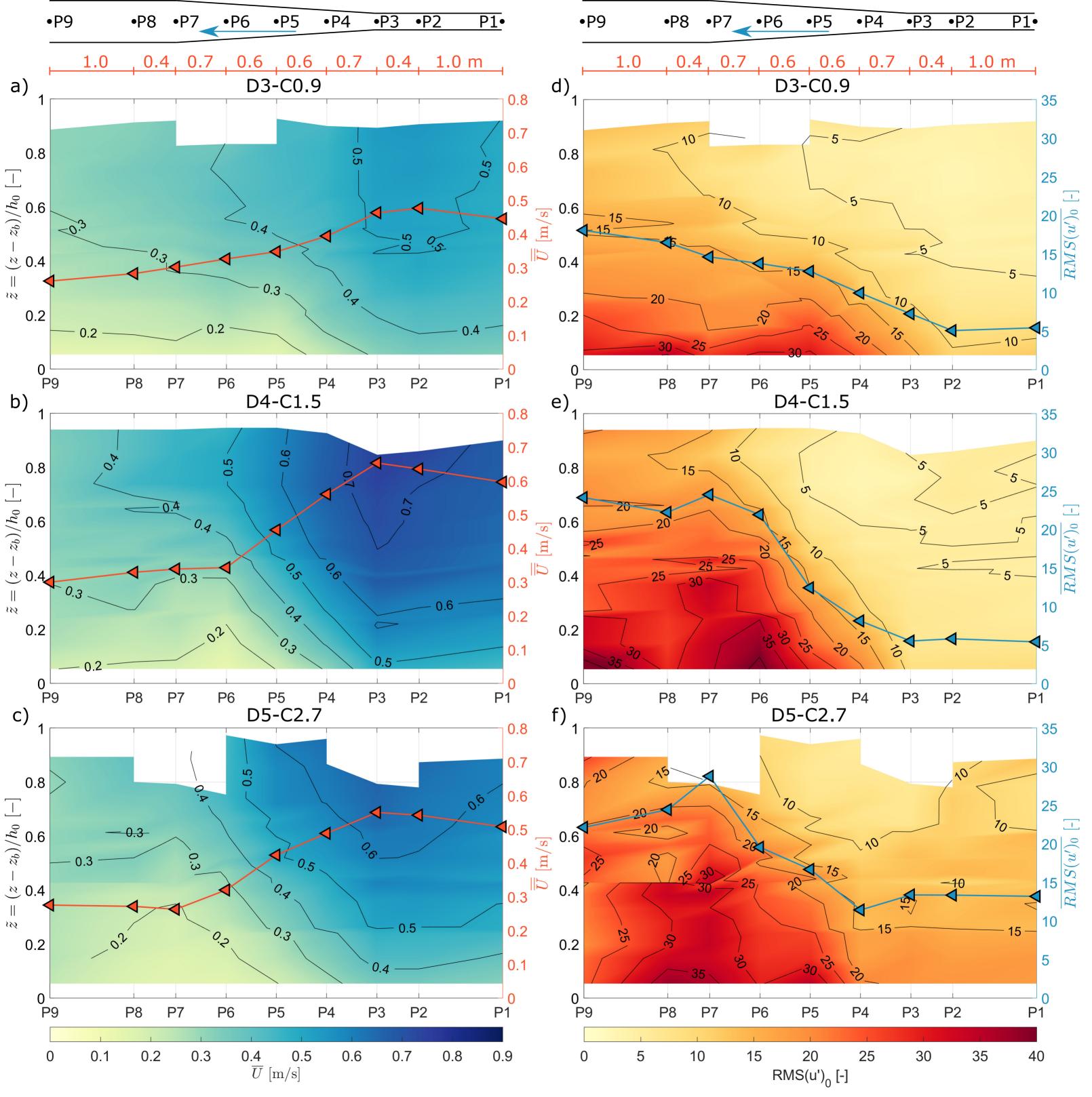
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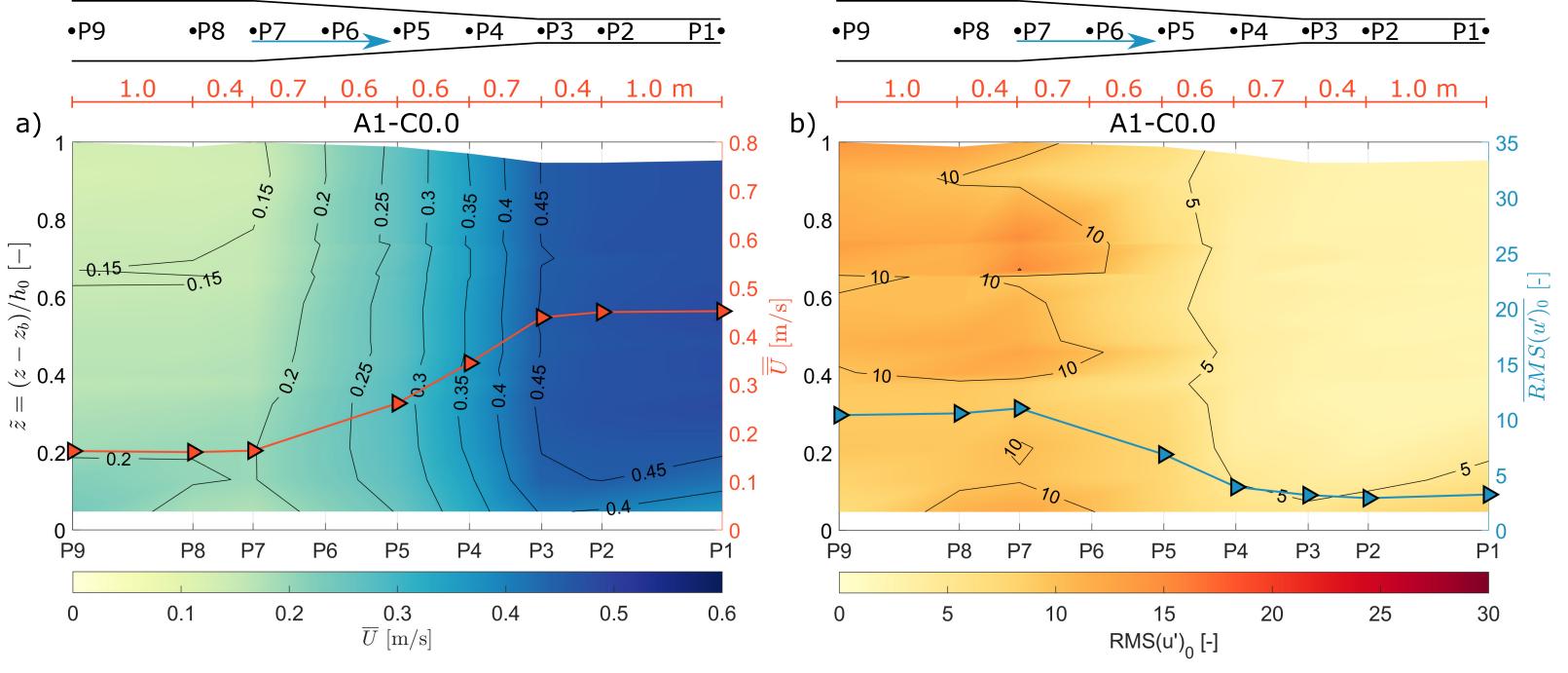
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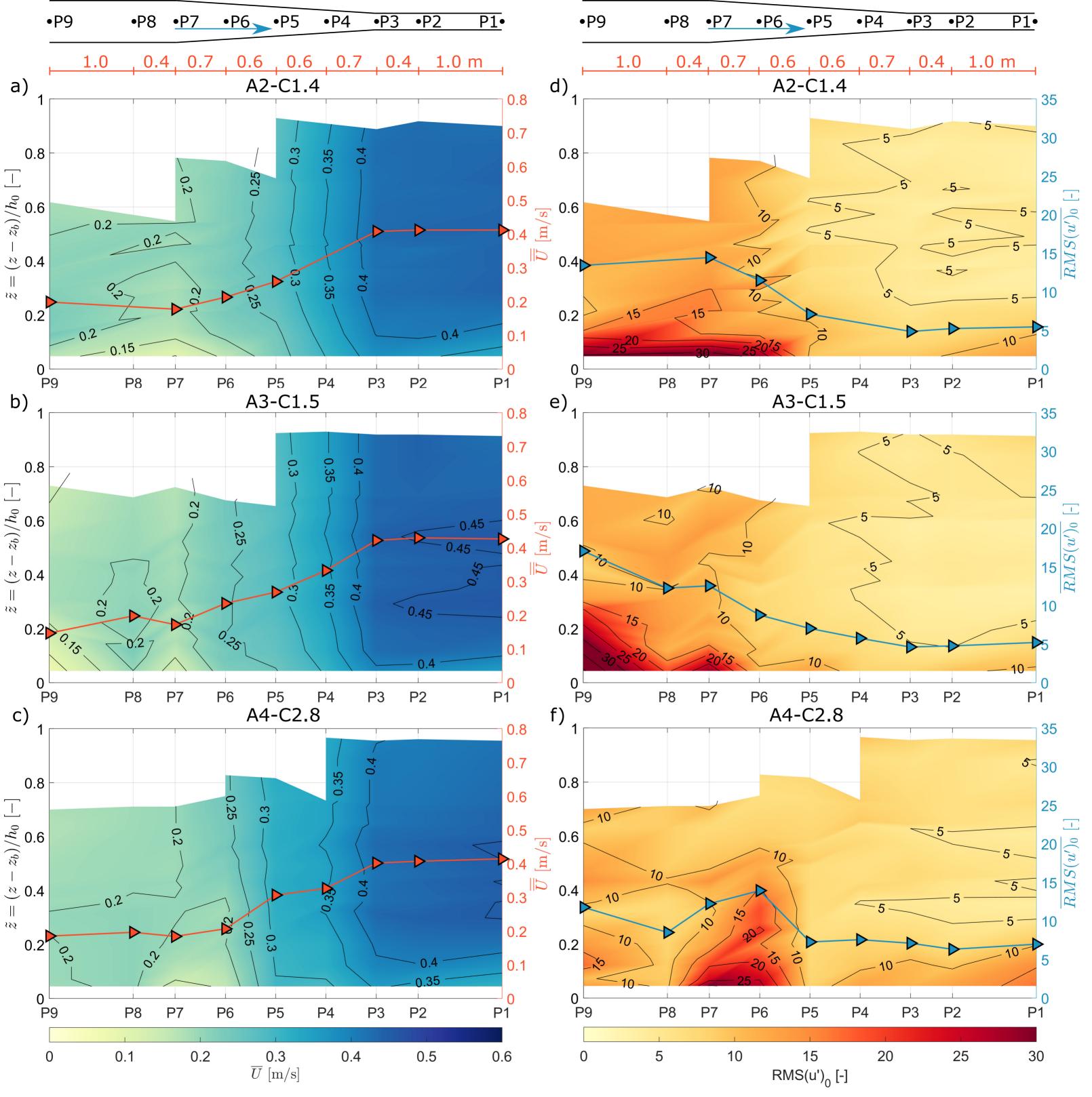
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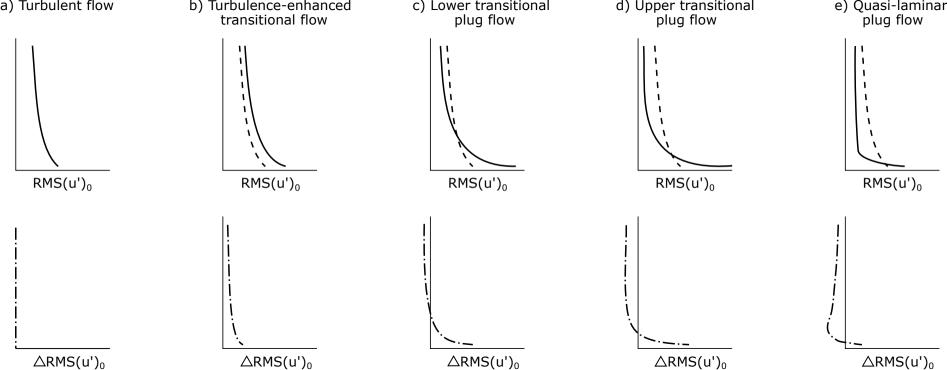
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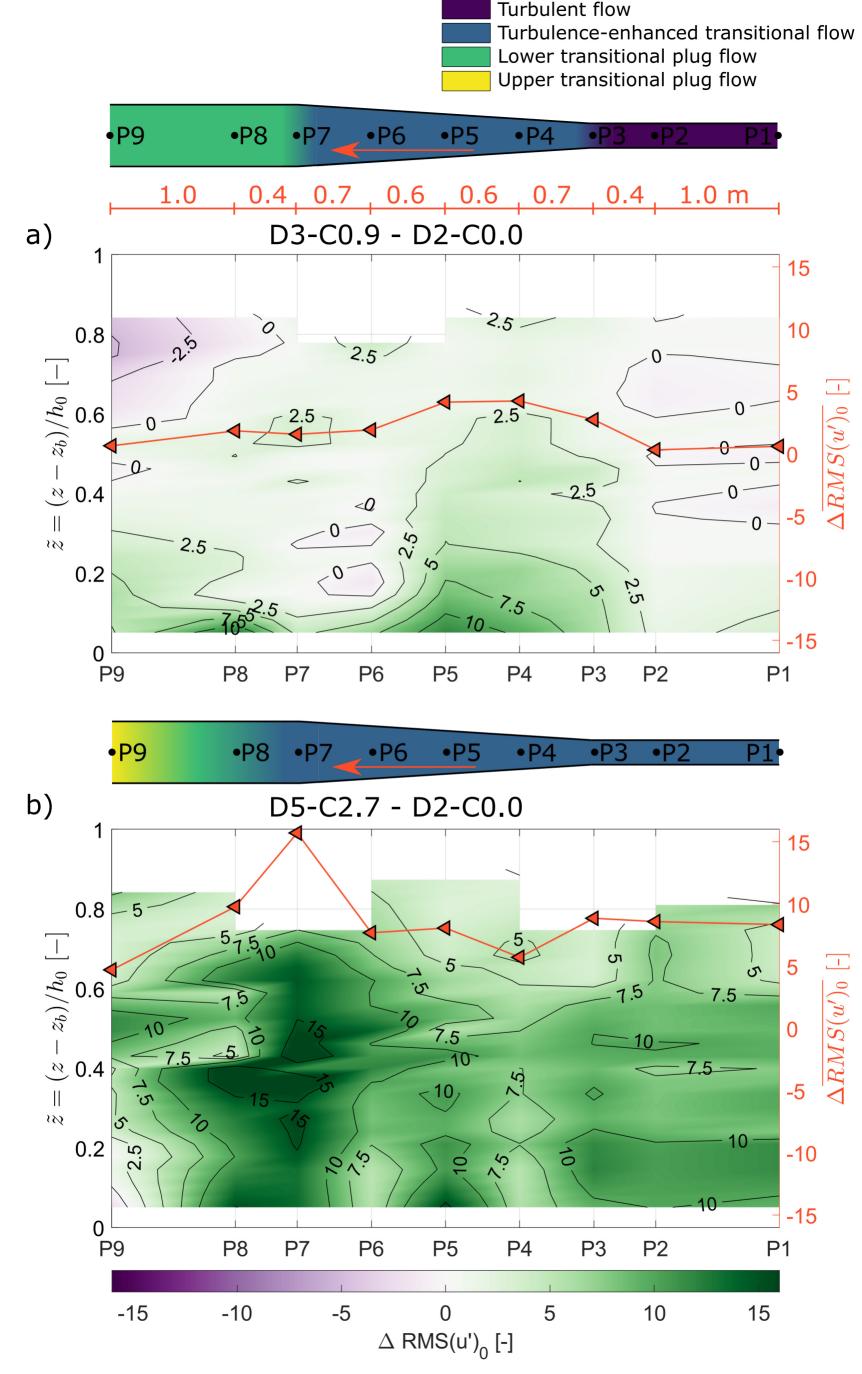
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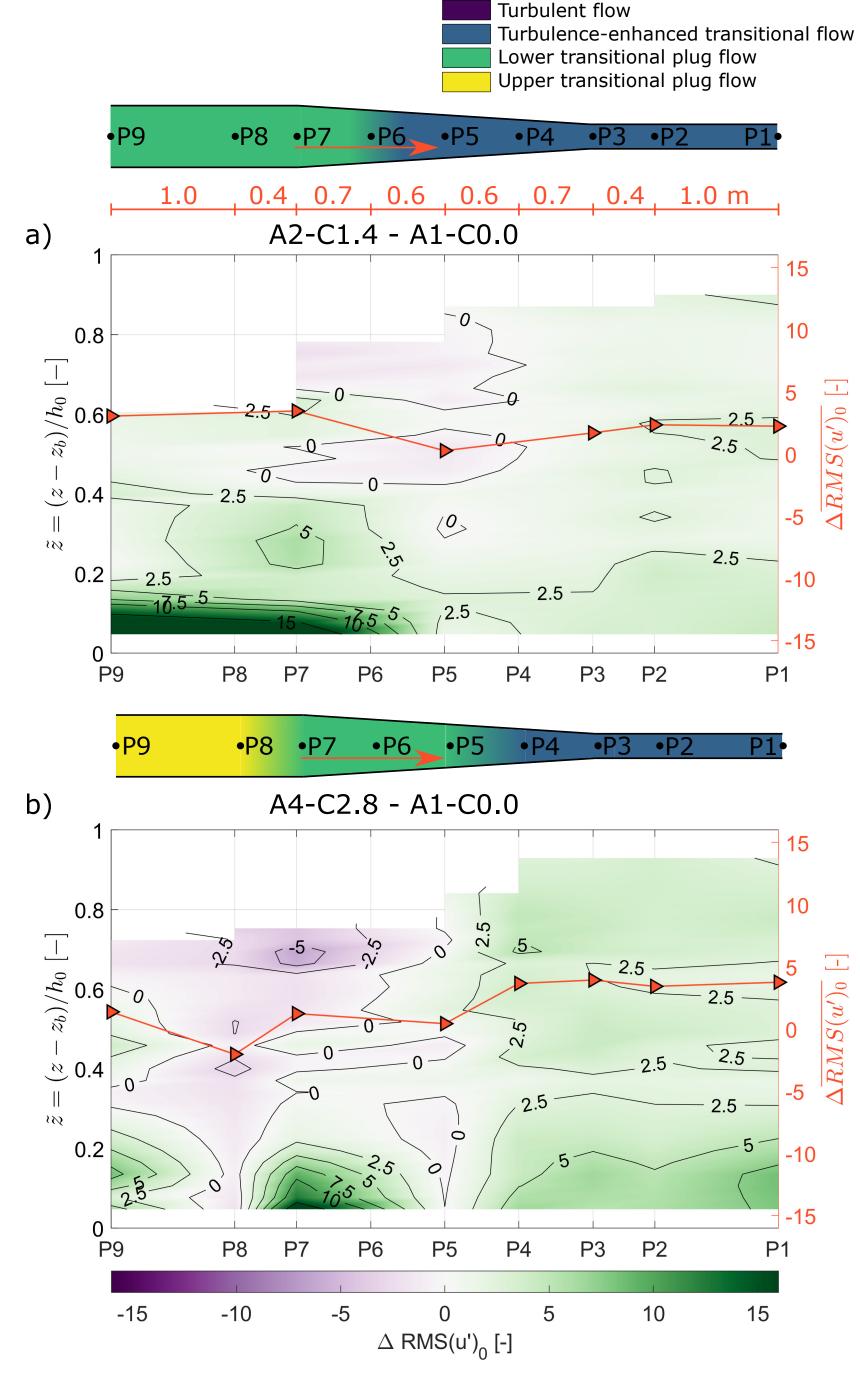
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